ISWRRI-87

COMPLETION REPORT

TD

224

.18

189

no.87

1977

TECHNICAL AND REGULATORY STATUS OF THE PLACEMENT OF SANITARY LANDFILLS IN IOWA

supported under the WATER RESOURCES RESEARCH ACT OF 1964 U. S. PUBLIC LAW 88-379 as amended

Lyle V. A. Sendlein, Principal Investigator James Kipp, Graduate Assistant

DEPARTMENT OF ENVIRONMENTAL QUALITY

Iowa State Water Resources Research Institute (ISWRRI) Iowa State University, 355 Town Engr. Bldg.

September 1977



COMPLETION REPORT

TECHNICAL AND REGULATORY STATUS OF THE PLACEMENT OF SANITARY LANDFILLS IN IOWA

Project No. A-065-IA¹

Duration: July, 1976 - September, 1977

Iowa State Water Resources Research Institute

Date of this Report: September, 1977

Lyle V. A. Sendlein, Principal Investigator



- - - ·

James Kipp, Graduate Assistant

IOWA STATE UNIVERSITY

Ames, Iowa 50011

Office of Water Research and Technology Agreement No.: 14-34-0001-7034

1/ Project financed in part by a grant from the U.S. Department of Interior, Office of Water Research and Technology, under Public Law 88-379, as amended, and made available through the Iowa State Water Resources Research Institute

ABSTRACT

This project includes a "state of the art" summary of landfill placement in Iowa and an update of ground water monitoring for three upland sites in Iowa. Major conclusions include:

- Current regulations favor upland placement of landfills, however, they occur in all topographic positions.
- 2. Pollution enclaves formed from floodplain sites are predictable and if this location is to be used the number in any one drainage basin must be considered to protect river water quality.
- 3. Pollution enclaves formed from upland sites are less predictable because of deeper penetration and larger number of geologic units encountered from source to sink. These sites may produce smaller volumes of
 - leachate than flood plain sites.
- 4. 53.1% of all sites are in the uplands, 41.5% are located on valley walls or ravines and only 5.4% occur on flood plains.
- 5. Approximately 35% of permitted landfills have required monitoring. It is interesting to note that 26% of all landfills occur in the high hazard zones of the Iowa Geological Survey classification and only 52% of them are monitored and 33% occur in the no hazard zone and 23% of them are monitored.

6. A high correlation exists between area of each hazard zone and the percent of landfills found in each area. This suggests that factors other than geologic control their location.

Data gathered from three upland sites are somewhat limited but several conclusions can be reached.

- Leachate concentrations are lower than those found in previous studies in Iowa on flood plain sites.
- 2. Because more activities of man can be encountered between the landfill and the ground water sink, the pollution enclave is more complex in its shape and direction of flow.
- 3. The production of leachate is more dependent on precipitation than flood plain sites. This means that the length of time over which leachate will be produced is lenthened.

Key words: ground water pollution, sanitary landfill, hydrogeology of landfills. -- SUMMARY --

This project was divided into two parts, 1 to write a "state of the art" report on landfill placement in Iowa and, 2 to monitor the effect of placement of landfills on upland sites. These topics are written up as two separate reports and are also summarized separately.

"State of the Art" report -- An inspection of all permits provided the information for tabulation and analysis of all sites in Iowa relative to topographic position, monitoring programs, geographic distribution, geologic position, river basins, and Iowa Geological Survey hazard zones. The following conclusions can be drawn on placement of landfills in Iowa.

CONCLUSIONS

General

1. There are 3 types of water resources which need to be

- considered with respect to their potential to be polluted by the disposal of solid waste. They are 1) surface water, 2) water in shallow unconsolidated aquifers, and 3) water contained in deeper bedrock aquifers.
- 2. Current regulations favor the placement of sanitary landfills in upland sites.
- 3. A knowledge of the relationships between geologic materials and water flow characteristics may allow for the prediction of contamination patterns which develop from landfills placed in any one of the topographic positions.

4. Pollution enclaves are plume-shaped with the long axis parallel to ground water flow lines. Concentrations decrease both with distance along the axis and with distance radially away from it such that the enclave is entirely surrounded by uncontaminated water except at its source.

Floodplain sites

- 1. The pollution enclave's shape and position are highly predictable in the alluvial materials near rivers.
- 2. The area underlain by contaminated ground water can be minimized by placing disposal sites adjacent to a river in a groundwater flow regimen where the flow lines are oriented normal to the river.
- 3. Rivers act as boundaries for leachate migration and are areas of discharge for it. Dilution is the principle method of attenuation.
- 4. Depth of penetration of the pollution enclave is minimized in a floodplain position due to the short distance of flow to a discharge point.

- 5. Ground water flow is controlled much more by river stage than by local amounts of precipitation in an alluvial system.
- 6. Enclave size in alluvium reaches a maximum size quickly and then is maintained in a steady-state equilibrium condition.
- 7. Water quality data suggests that contaminated ground water does not significantly affect the surface water into which it discharges.

8. The aggregate effect of a number of landfills in floodplain positions within a single river basin must be considered. The cumulative amounts of pollution added should not increase pollution concentrations within the river beyond certain limits.

Upland Sites

- The shape and position of pollution enclaves from upland disposal sites are more difficult to predict due to the natural variability of materials and the great distance to points of ground water discharge.
- The area underlain by contaminated water, therefore, has the potential to be very large in upland sites.
- 3. The distance to points of discharge are greater, and consequently the processes of sorption, decay, and filtration have a greater chance to reduce the concen-

tration of leachate from upland sites before it discharges into surface waters. Upland sites, however, are also regional and local recharge areas for bedrock aquifers.

- 4. The depth of penetration of the pollution enclave may be great and may reach into valuable bedrock aquifers.
- 5. Ground water flow is controlled by the amount of precipitation and the rate of infiltration in upland sites. The range of permeability of the various media present is also important in determining rates and direction of flow.

- 6. Because the available moisture depends on precipitation only, the enclave forms much more slowly than those which develop from disposal sites in floodplains.
- 7. Limited initial data from upland sites indicates that leachate concentrations are lower than those found in the floodplain. This may be due both to topographic position and the relative ages of the sites tested.
- 8. The long term effects of upland sites on water quality are not fully understood at this time.

Status of Iowa Landfills

- The effects of current state regulations favoring the placement of sanitary landfills in upland positions is evident (only 5.4% occur in the floodplain and terrace positions combined).
- 2. Approximately 35% of the permitted landfills have required monitoring programs.

The percentage of the total number of landfills in the state occurring in any one IGS hazard zone is closely related to the area of that zone. This reflects both population distribution and the cost of transportation as an important economic variable in waste disposal.
Many sites having non-required monitoring are potential sites for special management practices.

Effect of Upland Sites on Ground Water Quality --

CONCLUSIONS

Although the data generated from the current upland site studies are somewhat limited, several conclusions can be reached:

- 1. The shape and position of pollution enclaves from upland disposal sites are more difficult to predict than flood plain sites due to the natural variability of materials and the large distance to points of ground water discharge. Other activities of man between an upland landfill and the ground water sink may also be responsible for the reduction of water quality.
- 2. The production of leachate in upland positions is dependent on the amount and distribution of precipitation. Enclaves from upland sites therefore form more slowly than those which develop from disposal sites in floodplains where groundwater flow is more continuous. Leachate production from upland sites should be periodic, with high concentrations associated with flushing following major precipitation events.
- 3. The theoretical depth of penetration of pollution en-

5

claves from upland sites is greater than those associated with floodplains. The present data, in general, suggests that the concentration of leachate is greatest in the refuse and in nearby shallow materials. Water samples from deeper wells show that leachate concentrations decrease with distance away from the site and with increasing depth. This could be due to either a lack of production of significant amounts of leachate, or the attenuation of leachate as it moves through the ground.

 Limited initial data from the three upland sites indicate that leachate concentrations are lower than those found in previous investigations on floodplain sites (Peckenpaugh, 1973; Stevens, 1974). This may be a direct result of the topographic position with its associated ground water regimen. The relative ages of the sites tested may also be important. All of the upland landfills which were studied are fairly new and have been operated as sanitary landfills since the deposition of wastes was begun on the sites. The floodplain sites which were tested were much older and had a complex history of open dumping.

 PART ONE

TECHNICAL AND REGULATORY STATUS OF THE PLACEMENT

OF SANITARY LANDFILLS IN IOWA

TECHNICAL AND REGULATORY STATUS OF THE PLACEMENT OF SANITARY LANDFILLS IN IOWA

INTRODUCTION

Iowa is an area of humid continental climate. The citizens and industry of the state depend on ground water obtained from both near surface and bedrock aquifers. Because precipitation exceeds evaporation at some time during a normal year, excess rainfall infiltrates the surface and becomes part of the ground water system taking into solution various chemicals from solid wastes which have been deposited on or within the ground. In this hydrogeologic environment critical decisions must frequently be made regarding the placement and operation of solid waste disposal sites. Special problems are often encountered at individual sites with respect to the maintenance of protective systems and the implementation of monitoring programs to protect valuable ground water resources near solid waste disposal sites.

9

The location of landfills in marginal wet areas such as floodplains is generally discouraged due to the operational problems associated with these areas. The use of large and heavy equipment near the water table necessitates a high level of maintenance to lower ground water and remove surface water during the operation of the landfill. A commitment to a high level of maintenance must also be continued following closure of the site to protect important water resources. Landfills located in upland areas generally require much less maintenance for their operation, but may pose special problems with respect to the protection of water quality.

RESULTS OF STUDIES

Past research of sanitary landfills in Iowa and the potential pollution of groundwater from these sites has lead to cooperation between the Iowa State Water Resources Research Institute (ISWRRI), Iowa Department of Environmental Quality (DEQ), and the Iowa Geological Survey (IGS). These efforts contributed to the formation of rules and regulations governing sanitary landfills, and more recently to the updating of these regulations. Studies conducted by ISWRRI have considered the pollution released from existing landfills located on floodplains as well as those located in upland sites.

The research on sanitary landfills located on floodplains lead to several conclusions relative to their placement in this position. The data obtained suggests that the pollution enclaves which develop in alluvium are plume-shaped with the long axis parallel to the ground water flow lines. Research by both Drake

(1972) and Palmquist and Sendlein (1975) shows that the flow direction is generally directly from the landfill sites to the adjoining river. Decreasing concentrations were noted with distance along the axis. A general decrease in concentration both laterally and vertically away from the core of the enclave was also found. This means that an enclave is entirely surrounded by uncontaminated water except at its source.

An effort to use the electrical resistivity method to detect the pollution enclaves from floodplain landfills proved to be somewhat unsuccessful (Klefstad, 1973 and Klefstad, Sendlein and 11

Palmquist, 1975). As the inhomogeneity of the deposits in this topographic position increases, higher concentrations of ions in the leachate are needed for their detection because the natural scatter of resistivities due to the various media present mask the enclave anomaly. For this reason sample wells were installed with nests to achieve a vertical sampling distribution at each of the floodplain locations. The river stage was measured at each site, and groundwater evaluation and ion concentrations were tested for each well.

An analysis of the data obtained in this manner (Peckenpaugh, 1973 and Stephens, 1974) found:

- Rivers act as boundaries for leachate migration and are areas of discharge for it.
- Little mixing or dilution of leachate occurs after its initial introduction to the ground water.
- 3. The dominant control on concentrations is the water table condition and ground water flow.
- Ground water flow is controlled much more by river stage than by precipitation.
- Enclave size does not change in a uniform manner with time. Usually they achieve their maximum size quickly and then exist in a steady-state equilibrium condition.

The area underlain by contaminated ground water can be minimized by placing disposal sites adjacent to a river in a ground water regimen where flow lines are oriented normal to the river.

The water quality data (Palmquist and Sendlein, 1975) suggests that contaminated ground water does not significantly affect the surface water into which it discharges. Floodplain sites may be desirable in some cases where bedrock aquifers are vulnerable to contamination and are widely used as sources of water in the area. This is because of the predictability of 'the enclave shape and position in alluvium, the fact that floodplains are ground water discharge sites, and the low concentration of leachate formed in a high water table and high flow environment.

The initial study of upland sites as reported by Fenton (1973) proved to be inconclusive. In the spring of 1975 two new landfills in upland positions were instrumented by the ISU Geology group, one in Scott County and one in Fayette County. During the summer of 1976 an additional site in Cerro Gordo County was instrumented to monitor ground water quality. These three fills are currently being sampled on a monthly basis to evaluate their long term effects on ground water quality. Current regulations favor the placement of sanitary landfills in upland sites and these three sites represent this hydrogeologic condition. Data obtained from these sites is still somewhat limited, but to date the concentration

12

of the leachate which has formed appears to be lower than that associated with floodplain sites. More time is needed for a final analysis to be made.

Theoretically the predictability of pollution from upland sites is less than that of floodplain sites. This is due to the increased complexity of materials as well as the greater horizontal and vertical distances to areas of discharge. The size and depth of penetration of the pollution enclave will be greater and it may extend into underlying bedrock aquifers. The increased distance and time of travel before surface discharge should allow for greater attenuation by the processes of soption, decay, and filtration. The presence of jointed and cavernous rocks near the surface will reduce the amount of attenuation which occurs.

Separate studies by Kunkle (1976) and Kaufman (1970a, 1970b) of landfills in upland areas developed in glacial till show that the effect of these systems on ground water quality is not fully understood and indicate that bedrock aquifers may be affected after just 3-5 years of landfill activity. It is apparent that the analysis of the effects of upland sites on groundwater quality should be continued. This research may lead to the re-examination of current governmental policy and provide a basis for recommending the revision of present regulations governing the placement of sanitary landfills.

The DEQ is responsible for issuing permits to agencies desiring to operate landfills in the state. The DEQ works in conjunction

13

with the IGS to determine whether proposed sites meet current regulations and protect the natural resources and environment of the state. The applicant must provide detailed information regarding geology, ground and surface water hydrology, and plans for the design and operation of the site. The permit section reviews each application in cooperation with the IGS, which provides information relating to the geology and water use of the area.

The Iowa Department of Environmental Quality recognizes 5 general types of waste management sites in the administration of its permit program. They include sanitary landfills, construction and demolition debris sites, compaction and/or transfer stations, resource recovery units, and composting operations. Permits typically run for 3 years during which the DEQ continues to monitor the site in a surveillance program to insure that the original specifications of the permit are maintained. Special provisions can be included in granting a permit to require the monitoring of ground water quality in the area. Other management practices may also be required as a condition of the permit.

This report is an attempt to assemble the available information concerning the permitted landfills in Iowa and to analyze the effects of current governmental policy on their placement. The position of each landfill in terms of hydrogeologic environment is presented. Means of controlling landfill leachate production and the implementation of protective systems and monitoring programs in the state are also given.

METHOD OF STUDY

14

Data Source

During July and August of 1976 the DEQ permit files and engineering specifications were studied to obtain the information available at that time concerning the placement and operation of each landfill in Iowa. In June of 1977 the data was updated to include all permits granted by the DEQ since the inception of the permit program. An additional effort was made to classify each site with respect to topographic position, zone of hazard with respect to ground water contamination, and major drainage basin. Topographic Classification

The topographic classification used in this report is based on research conducted during the past 15 years by a number of individuals. Toth (1963) analyzed the ground water flow of a local drainage basin. He found a local hydrologic system consisting of recharge points at minor topographic highs and discharge points at adjacent topographic lows. An intermediate hydrologic system consists of recharge and discharge points which are not necessarily adjacent to or at the extreme topographic positions. The regional flow system involves recharge at ground water divides with discharge into the bottom of the basin.

Using this model the groundwater flow of a region can be divided into three zones. The upper zone is active and consists of flow with a base level equal to the level of small streams. The middle zone is characterised by delayed flow which is subject to

15

less climatic effect. Base level of this zone is equal to the bottoms of the largest rivers. A lower zone containing relatively stagnant water exists in a position beneath the level of the largest river (Figure 1).

LeGrand (1965) stated that an understanding of the relationships between geologic materials, ground water, and surface water could be used for the selection of landfill sites. He proposed the shape of a contamination enclave as a flame-shaped plume with its axis oriented parallel to the ground water flow lines (Figure 2). Palmquist and Sendlein (1975) noted that if a source of recharge by the infiltration of precipitation is considered, a zone of cleaner



homogeneous material (after Toth, 1963).



FIGURE 2. Plan view of a water table aquifer showing the hypothetical areal extent to which contaminants disperse and move to insignificant levels within the contamination enclave (after LeGrand, 1965). water exists on top of the polluted water. Thus the contaminated ground water is a three-dimensional body formed at a source and extenting from it parallel to the flow pattern, through uncontaminated water, to a discharge point (Figure 3).

The topographic position when added to this information and considered in light of Toth's flow model may enable the prediction of the contamination pattern for landfills placed in any topographic position. A hydrogeologic model for this purpose has been proposed (Sendlein and Palmquist, 1973 and 1975). In this model the landscape was divided into seven positions: 1) floodplain, 2) terrace, 3) valleyside, 4) ravine, 5) convex crest, 6) upland valleyside, and 7) upland flat (Figures 4 and 5). This system was used to classify the topographic position of each of the permitted landfills in Iowa from the available data.

The current law in Iowa assumes that a sanitary landfill

17

location which minimizes the flow of water through the refuse is prefered. Under these regulations the most suitable sites are the upland, crest, and upper gully. Using Toth's flow model it can be seen that the upland sites could have a high potential for contamination of major bedrock aquifers due to their location in local or regional recharge areas. The floodplain and river terrace sites may be more acceptable with respect to this criterion because they occur at discharge points where pollutants are rapidly released to the adjacent stream where they may be diluted to acceptable concentrations. In light of all of this information the landfills of Iowa were classified as to their topographic position.





FIGURE 4. Topographic classification of potential disposal sites within a landscape (Sendlein and Palmquist, 1975).



19

Classification With Respect to IGS Hazard Zones

The Iowa Geological Survey has prepared a report concerning the placement of sanitary landfills in the state (Tuthill and others, 1972). In this analysis 3 water resource types have been recognized: surface water, groundwater in shallow unconsolidated sediments, and water contained in numerous bedrock aquifers. Iowa can be divided into 3 regions based on the similarities in use of their water resources (Figure 6).

The Eastern Iowa Groundwater District includes approximately the eastern one-half of the state. In general, carbonate aquifers are shallow and are often the uppermost geologic unit. Glacial drift and loess are thin or absent, making the shallow bedrock aquifers widely used as a source of water.

The Southern Iowa Groundwater District is underlain by bedrock sources of poor quality. In this region most of the potable water is obtained from runoff, shallow alluvial deposits, or surface water.

20

Contamination of the bedrock is therefore of less importance with respect to water supply.

The Western Iowa Groundwater District is underlain by a sandstone aquifer containing water of variable quality. The use of surface water and alluvial or loess aquifers is locally important. The general thickness of the overburden materials as well as the limited use of groundwater from the bedrock aquifers makes this region a zone of low hazard with respect to pollution of the bedrock aquifer.

When the information on the types of water resources used in these regions is superimposed over the bedrock geologic map of the

BEDROCK OF IOWA

LEGEND

RETACEOUS

Ku Undifferentiated

URASSIC

Jid Fort Dodge Beds -

ENNSYLVANIAN

Virgil Pv Pm

Missouri

Pdm Des Moines

AISSISSIPPIAN

Undifferentiated

EVONIAN

Du Upper

Dm Middle

ILURIAN

Su Undifferentiated

RDOVICIAN

Ou Undifferentiated

10

0

30 miles

20

10

AMBRIAN

£ш

Undifferentiated

RECAMBRIAN

Crystalline oCc





FIGURE 6. Groundwater Districts of Iowa

Williams & Heintz Map Corporation, Washington, D.C.

(AFTER TUTHILL AND OTHERS, 1972) state four zones of hazard with respect to landfill site selection are obtained (Figure 7).

Zone A is a high hazard zone underlain by shallow bedrock aquifers which may be fractured and cavernous. Special care must be taken in this area to protect the aquifers most widely used in the region. Zone B is a moderate hazard zone. Sources of water may come from surface water or shallow unconsolidated and bedrock aquifers. Less care is therefore required in the protection of bedrock aquifers, but care should be taken to protect surficial aquifers. Zone C is a low hazard zone usually having bedrock aquifers protected by units of low permeability shale. Generally overburden is thick and allows some time for the attenuation of pollution. Zone D is termed a no hazard zone. Most water used in this area comes from runoff and surface waters so protection of subsurface water is of relatively minor importance as compared to the other regions.

It is important, therefore, to know the position of each landfill in the state with respect to the hazard it produces to the water resources used in that region. The permitted landfills consequently are classified as to the zone of hazard in which they occur.

Classification With Respect to Surface Drainage Basin Because certain areas of the state rely on runoff and surface waters as a source of supply, an attempt to divide the state into

its major drainage basins was made. The classification used was



Landfill Site Selection.

and others, 1972)

adopted from Bulletin Number 7 of the Iowa Highway Research Board, titled <u>Drainage Area of Iowa Streams</u>. Each permitted landfill was classified as to the major drainage basin in which it is included (Figure 8). This will aid in the analysis of the distribution of landfills in the state.

ANALYSIS AND DISCUSSION

An analysis of the topographic placement of the landfills in Iowa reveals the effects of current state regulations favoring upland sites (Table 1). Only 5.4% of all landfills in the state are located in the low topogrpahic positions (floodplains and terrace). All fills in these low areas are required by their operational permits to maintain monitoring programs. This is in contrast with the remainder of the disposal sites in the state of which only about one-third have required monitoring programs.

The percentage of landfills with required monitoring seems

to vary between the Iowa Geological Survey hazard zones with respect to landfill placement (Table 2). The high hazard zone (A) has the greatest proportion of its sites with required monitoring, but beyond that there appears to be no real pattern. The total number of landfills in each hazard zone is also somewhat variable. The zone of no hazard (D) has the highest number of landfills. If an effort to place landfills in the safest positions was being made this would be expected. Close inspection, however, shows that the high hazard zone (A) has the second highest number of landfills. This means that there must be another explanation for the placement

MAJOR DRAINAGE BASINS



(After Iowa Highway Research Board, Bulletin No. 7) FIGURE 8.

\mathbf{a}	0	
	6	
4	U	

TABLE ONE:

LANDFILLS BY TOPOGRAPHIC POSITION

POS	ITION	NUMBER	% OF TOTAL	% WITH REQUIRED MONITORING
1.	Floodplain	1	1.1	100
2.	Terrace	4	4.3	100
3.	Valleyside	11	11.7	36.4
4.	Ravine	28	29.8	25
5.	Convex Crest	7	7.4	28.6
6.	Upland Valleyside	34	36.2	41.2
7.	Upland Flat		9.6	22.2
		94		

TABLE TWO: LANDFILLS BY IGS HAZARD ZONE

ZONE	NUMBER	% OF TOTAL	% WITH REQUIRED MONITORING
A HIGH	25	26.6	52
B MODERATE	13	13.8	23.1
C LOW	25	26.6	36
D NO	31	33	22.6

of landfills in the state. The variable pattern of the number of landfills within each hazard zone makes little sense unless compared to the total area of each zone. It is clear that the percentage of the landfills in the state for each hazard zone is closely related to the area of that zone (Table 3). This reflects the cost of transportation as an important variable to be considered in waste disposal. Thus there is a definite limited economic area that can be served by a single landfill, the limiting factor being transportation costs. This would tend to keep the number of landfills per unit area approximately equal.

A study of the distribution of landfills in the drainage basins of the state shows that over one-half of the landfills are found in only 3 basins (Table 4). These basins, however, are large and occupy about one-half of the area of the state. This again points to the fact that the average landfill density throughout the

27

state is nearly constant and is related to the cost of transportation. Table 5 shows a division of landfills with required monitoring by topographic position in each hazard zone. Table 6 shows the total number of landfills in each topographic position that have required monitoring. The upland valleyside position has the greatest number of sites with monitor programs. This is due to the large number of fills in this position (36% of total in state) and is also related to an attempt to safeguard the surface water in small upland streams with limited dilution capacities. Table 7 shows the percentage of landfill sites with required monitoring by topographic zone.

TABLE THREE: AREA OF IGS HAZARD ZONES (approximate)

ZONE	% OF AREA OF STATE	% OF TOTAL LANDFILLS
A HIGH	27	26.6
B MODERATE	16	13.8
C LOW	27	26.6
D NO	30	33.0

TABLE FOUR:	SANITARY	LANDFILLS	BY D	RAINAGE	BAS	SIN
RIVER BASIN				NUMBER	IN	BASIN
Des Moines (and Raccoon)					23	
Cedar (Fox, Iowa, & Englis	h)				19	
Skunk					9	
Nishnabotna					5	
Missouri					5	
Turkey					4	
Little Sioux				i dalla	4	
Maquoketa					3	
Wapsipinicon					3	
Platte					3	
Thompson					2	
Boyer					2	

1 Nodaway 1 Chariton Soldier 1 1 Rock 1 Mosquito 1 Floyd 1 Upper Iowa 1 UNKNOWN

TABLE FIVE: TOPOGRAPHIC POSITION OF REQUIRED MONITORING BY HAZARD ZONE

ZON	NE A (HIGH HAZARD)	
Position		Number Required
1.	Floodplain	1
3.	Valleyside	1
4.	Ravine	1
5.	Convex Crest	2
6.	Upland Valleyside	5
7.	Upland Flat	3
		13

(One demolition debris site in this zone is also required to have monitor wells.)

1

2

1

1

4

3

2

2

1

2

ZONE B (MODERATE HAZARD)

- 4. Ravine
- 6. Upland Valleyside

30

ZONE C (LOW HAZARD)

- 2. Terrace
- 3. Valleyside
- 4. Ravine
- 6. Upland Valleyside

ZONE D (NO HAZARD)

- 2. Terrace
- 3. Valleyside
- 4. Ravine
- 6. Upland Valley

TABLE SIX: REQUIRED MONITORING BY TOPOGRAPHIC POSITION

POSITJON NUMBER REQUIRED Floodplain 1. 2. Terrace 4 Valleyside 3. 4 Ravine 7 4. 5. Convex Crest 2 12 Upland Valleyside 6. Upland Flat 7. 3 33

TABLE SEVEN: % OF LANDFILLS WITH REQUIRED MONITORING BY TOPOGRAPHIC ZONE

% WITH REQUIRED MONITORING

TOPO ZONE

Lowland (floodplain and terrace)

100

28.2 Valleyside (and ravine) 34

Uplands (crest, valley, flat)

Some disposal sites have required monitoring, but no well monitoring network is used. These areas generally have surface water or existing wells and tile systems which are required to be monitored and are listed in Table 3. Some landfills in the state are not required to carry on monitoring programs, but have the capability to do so. Three permitted fills are included in this category and are shown in Table 9.

Only one landfill in the state of Iowa was constructed using the synthetic lining system. This fill is used for the disposal of foundry wastes from the John Deere plant in Dubuque. The area of the landfill is underlain by high permeability loess over carbonate bedrock. With the use of this synthetic liner (produced by B. F. Goodrich under the trade name "Flexseal") leachate is collected and used for spray irrigation during the summer months. During the remainder of the year lechate is collected and taken to a wastewater treatment plant. This permit is shown in Table 10.

32

Many of the fills in the state have management systems which may lend themselves to future leachate collection and treatment. Generally these sites have some type of ground water interceptor tile. These tile lines may be very useful in the future if a serious leachate problem develops. The landfills with tile lines proposed or in operation are listed in Table 11. The DEQ also grants permits for alternative types of waste handling procedures. Other activities controlled by the sanitary

landfill permit system include transfer stations, incinerators, recycling centers, construction and demolition debris disposal sites, and garbage composting facilities. The agencies permitted to
OTHER REQUIRED MONITORING

COUNTY	PERMIT NUMBER	TYPE OF MONITORING
Appanoose	4-SDP-1-76P	Existing pond and sludge drying lagoon
Cedar	16-SDP-1-76P	Existing pond
Clinton	23-SDP-1-74P	Existing well plus three new ones
Clinton	23-SDP-2-74P	Existing bedrock wells in Galena Formation
Crawford	24-SDP-1-73P	Existing tile system
Iowa	48-SDP-1-75P	Existing tile system
Mahaska	62-SDP-1-74P	Existing wells
TABLE NINE:	LANDFILLS WITH NON-RE	QUIRED MONITORING
Des Moines	29-SDP-1-76P	Two wells fitted with perforated PVC
Hamilton	40-SDP-2-75P	One well in
Man Duran	00 CDD 1 75D	The wells fitted with

Van Buren

TABLE EIGHT:

80-SDP-1-75P

Two wells fitted with perforated PVC

TABLE TEN: LANDFILLS WITH LINERS

Debuque (John Deere) 31-SDP-1-75P

B. F. Goodrich "Flexseal" liner; Leachate collection for summer spray irrigation and winter sewage treatment. TABLE ELEVEN: LANDFILLS WITH LEACHATE MANAGEMENT POTENTIAL

COUNTY	NUMBER	MANAGEMENT CAPABILITY
Benton	6	Tile collection
Black Hawk	7	Berm for collection
Boone	8	Diversion tile
Calhoun	13	Tile collection
Cerro Gordo	17	Tile collection
Crawford	24	Tile collection
Dallas	25	Slurry trench
Dubuque	31.2	Tile collection
Franklin	35	Tile collection
Greene	37	Dam to catch surface seeps
Grundy	38	Underground slurry trench
Iowa	48	Tile collection
Jackson	49	Cut-off dikes for surface runoff

Jones	53	Berm to control leachate
Lee (Fort Madison)	56.1	Dam at end of ravine to collect surface water
Madison (Rice)	61.1	Tile collection
Marshall	64.2	Ground water interceptor
Muscatine	70.2	Subsurface dike to control leachate migration
Polk (Metro West)	72.2	Berm keyed to till
Webster	94	Collection plan

perform these activities in the state of Iowa are listed in Table 12.

Table 13 is a list of all permitted waste handling facilities in the state of Iowa. Those landfills with required monitoring are included in Table 14.

CONCLUSIONS

General

- There are 3 types of water resources which need to be considered with respect to their potential to be polluted by the disposal of solid waste. They are 1) surface water, 2) water in shallow unconsolidated aquifers, and 3) water contained in deeper bedrock aquifers.
- Current regulations favor the placement of sanitary landfills in upland sites.

- 3. A knowledge of the relationships between geologic materials and water flow characteristics may allow for the prediction of contamination patterns which develop from landfills placed in any one of the topographic positions.
- 4. Pollution enclaves are plume-shaped with the long axis parallel to ground water flow lines. Concentrations decrease both with distance along the axis and with distance radially away from it such that the enclave is entirely surrounded by uncontaminated water except at its source.

TABLE TWELVE:

SPECIAL PERMITS

TRANSFER STATIONS

COUNTY	PERMIT NUMBER
Clay	21-SDP-1-76P
Palo Alto	74-SDP-1-76P
Pocahontas	76-SDP-1-75P
Polk	77-SDP-5-75P
Polk (fairgrounds compactor)	77-SDP-6-75P
Polk (Public Service Trucking)	77-SDP-8-75P
Polk (Metro Transfer Station)	77-SDP-10-76P
Scott	82-SDP-1-75P

INCINERATORS

Johnson	52-SDP-2-76	
Story (heat recovery)	85-SDP-2-75	

RECYCLING

Des Moines (Pak-A-Way) Linn 29-SDP-2-76P 57-SDP-2-74P

Polk (Metro Recycling)

77-SDP-3-73P

CONSTRUCTION AND DEMOLITION DEBRIS

Linn 57-SDP-1-72P* Marshall 64-SDP-1-75P* Muscatine 70-SDP-1-74P Muscatine 70-SDP-3-76P Polk (J. C. White-Ovid) 77-SDP-7-75P* Polk (J. C. White-Market) 77-SDP-9-75P* Story 85-SDP-3-75P

DIGESTER

Polk (Enviro-Systems Ltd.)

77-SDP-4-74P

* Required Monitoring

GUIDE TO TABLES 13 & 14

The data in Tables 13 and 14 are arranged in the following way. The first column contains numbers corresponding to the county names arranged alphabetically. The second column has the permit number of each landfill within a given county arranged chronologically, the first permit granted having the number 1. Permit numbers issued by the DEQ are in the following format:

County Number-SDP-Fill Number-Issue Year P

Column 3 is the name of the river basin in which the landfill occurs. If monitoring is required by the DEQ, the word "yes" appears in column 4. The final column is the topographic position in which each fill occurs. A key to the method used is shown below:

1-floodplain

2-terrace

3-valleyside

4-ravine

5-convex crest

6-upland valley

7-upland flat



ALPHABETICAL LIST OF COUNTY NAMES

- 1. Adair 2. Adams 3. Allamakee 4. Appanoose 5. Audubon 6. Benton 7. Blackhawk 8. Boone 9. Bremer 10. Buchanan 11. Buena Vista 12. Butler 13. Calhoun 14. Carroll 15. Cass 16. Cedar 17. Cerro Gordo 18. Cherokee 19. Chickasaw 20. Clarke 21. Clay 22. Clayton 23. Clinton 24. Crawford 25. Dallas 26. Davis 27. Decatur 28. Delaware
- 29. Des Moines

Jefferson 51. 52. Johnson 53. Jones 54. Keokuk 55. Kossuth 56. Lee 57. Linn 58. Louisa 59. Lucas 60. Lyon 61. Madison 62. Mahaska 63. Marion 64. Marshall 65. Mills 66. Mitchell 67. Manona 68. Monroe 69. Montgomery 70. Muscatine 71. Obrien 72. Osceola 73. Page 74. Palo Alto 75. Plymouth 76. Pocahontas 77. Polk 78. Pottawatomie 79. Poweshiek

30.	Dickinson	
31.	Dubuque	
32.	Emmet	
33.	Fayette	
34.	Floyd	
35.	Franklin	
36.	Fremont	
37.	Greene	
38.	Grundy	
39.	Guthrie	
40.	Hamilton	
41.	Hancock	
42.	Hardin	
43.	Harrison	
44.	Henry	
45.	Howard	
46.	Humbolt	
47.	Ida	
48.	Iowa	
49.	Jackson	
50.	Jasper	

80.	Ringgold
81	Sac
01.	Sac
82.	Scott
83.	Shelby
84.	Sioux
85.	Story
86.	Tama
87.	Taylor
88.	Union
89.	Van Buren
90.	Wapello
91.	Warren
92.	Washington
93.	Wayne
94.	Webster
95.	Winnebago
96.	Winneshiek
97.	Woodbury
98.	Worth
99.	Wright

	T.	ABLE 13.	LANDFILLS	IN IOWA		
	1	2	3	4	5	6
CBS	CCUNTY	FILL	HAZARD	BASIN	MCNITOR	FOSITION
					NC C	
1	1	1	U	NUDAWAY	YES	0
2	4	1	U	CFARITUN	NU	3
3	5	1	C	NISHNABL	NU	6
4	6	1	A	CEDAR	NO	4
5	7	1	A	CEDAR	YES	6
6	8	1	D	DESMOINE	NC	4
7	9	1	A	TURKEY	YES	3
8	10	1	A	WAPSIPIN	YES	e
9	11	1	C	DESMOINE	YES	2
10	12	1	A	CEDAR	NO	7
11	13	1	D	CESMC INE	NC	7
12	14	1	D	DESMOINE	NO	6
13	15	1	C	NISHNABO	YES	6
14	16	1	Α	CEDAR	NC	4
15	17	1	Α	CEDAR	YES	6
16	18	2	С	LTLSICUX	NO	e
17	19	1	Α	WAPSIFIN	NC	3
18	20	1	D	DESMOINE	YES	4
19	20	2	D	THOMPSON	YES	2
20	23	1	Α	MISSISSI	NO	4
21	23	2	Α	WAPSIPIN	NO	5
22	24	1	С	BCYER	NO	4
23	25	1	D	DESMOINE	YES	3
24	27	1	D	THOMPSON	NO	5
25	28	1	A	MAQUCKET	YES	5
26	29	1	С	MISSISSI	NO	4
27	30	1	С	LTLSICUX	NO	6
28	31	1	A	TURKEY	NC	4
29	31	2	A	TURKEY	ND	4
30	32	1	в	DESMOINE	NO	6
31	33	1	A	TURKEY	YES	7
32	35	1	с	CEDAR	NO	6
33	36	1	D	NISHNABC	YES	6
34	37	1001	D	DESMOINE	NC	4
35	38	1	с	CEDAR	YES	6
36	39	1	C	DESMOINE	NO	6
37	40	1	в	DESMOINE	NO	7
38	40	2	8	SKUNK	NO	6
39	42	1	B	CEDAR	NO	7
40	43	1	D	SOLDIER	NO	6
41	44	1	в	SKUNK	YES	4
42	46	1	В	DESMOINE	NO	6
43	47	1	c	LTLSIOUX	NO	E
44	48	1	c	CEDAR	NC	4
45	49	1	A	MAQUCKET	NC	۵
46	50	1	D	SKUNK	ND	7
47	52	1	c	CEDAR	YES	4
48	53	1	A	MAQUOKET	NO	4
49	54	1	E	SKUNK	NO	4
50	55	1	В	DESMOINE	NC	6
51	56	1	C	SKUNK	YES	4
52	56	2	B	SKUNK	NO	5
53	56	3	C	MISSISSI	NG	4
54	57	3	A	CEDAR	YES	1

×.

		TABLE 13.	LANDFILLS	IN IOWA	(continued)	
	1	2	3	4	5	6
OBS	COUNTY	FILL	HAZARD	BASIN	MCNITCR	POSITION
55	58	1	C	CEDAR	YES	4
56	60	1	C	ROCK	NO	6
57	61	1	D	DESMOINE	NO	6
58	61	2	D	DESMOINE	NO	6
59	62	1	В	SKUNK	YES	6
60	63	1	D	DESMOINE	NC	2
61	64	2	D	SKUNK	NO	4
62	65	1	D	MCSQUITC	NC	7
63	66	1	Α	CEDAR	YES	7
64	67	1	С	LTLSICUX	NO	4
65	68	1	D	DESMCINE	NC	3
66	69	1	D	NISHNABO	NO	e
67	70	1	Α	CEDAR	NO	4
68	70	2	A	CEDAR	YES	4
69	73	1	D		NO	6
70	74	2	В	DESMOINE	YES	6
71	75	1	С	FLOYD	NC	6
72	7.7	1	D	DESMOINE	YES	3
73	77	2	D	DESMCINE	NC	5
74	79	1	C	CEDAR	NO	6
75	81	1	D	BCYER	NO	4
76	82	2	А	MISSISSI	YES	6
77	82	З	Α	MISSISSI	YES	5
78	83	1	D	NISHNABC	NO	5
79	84	1	С	MISSOURI	YES	6
80	84	2	С	MISSOURI	NO	3
81	85	2	D	SKUNK	YES	2
82	86	1	С	CEDAR	YES	3
83	87	1	D	PLATTE	NO	6
84	88	1	D	PLATTE	NO	4
85	88	2	D	PLATTE	NC	E
86	89	1	в	DESMOINE	NC	3
87	90	1	D	DESMOINE	NC	. 3
88	94	1	D	DESMOINE	NO	4
89	95	1	Α	DESMOINE	NC	7
90	96	1	A	UFIOWA	YES	6
91	97	1	С	MISSOURI	YES	4
92	97	2	с	MISSOURI	NO	. 4
93	98	1	A	CEDAR	YES	7
94	99	1	В	CEDAR	NO	7

	TABLE 14.	LANDFILLS	WITH REQU	IRED MONITOR	WELLS	
	1	2	3	4	5	6
085	COUNTY	FILL	HAZAPD	BASIN	MCNITOR	POSITION
1	. 1	1	D	NODAWAY	YES	6
2	7	1	Α	CEDAR	YES	6
3	9	1	A	TURKEY	YES	3
4	10	1	A	WAPSIPIN	YES	6
5	11	1	С	DESMOINE	YES	2
5	15	1	С	NISHNABO	YES	6
7	17	1	A	CEDAR	YES	6
8	20	1	D	DESMOINE	YES	4
9	20	2	D	THOMPSON	YES	2
10	25	1	D	DESMOINF	YES	3
11	28	1	А	MAQUOKET	YES	5
12	33	1	А	TURKEY	YES	7
13	36	1	D	NISHNABO	YES	6
14	38	1	С	CEDAR	YES	6
15	44	1	В	SKUNK	YES	4
15	52	1	с	CEDAR	YES	4
17	56	1	с	SKUNK	YES	4
18	57	3	A	CEDAR	YES	1
19	58	1	С	CEDAR	YES	4
20	62	1	в	SKUNK	YES	6
21	66	1	A	CEDAR	YES	7
22	70	2	А	CEDAR	YES	4
23	74	2	в	DESMOINE	YES	6
24	77	1	D	DESMOINE	YES	3
25	82	2	A	MISSISSI	YES	6
26	82	7	A	MISSISSI	YES	5
20	84	1	C	MISSOURI	YES	6
20	85	2	D	SKUNK	YES	2
6 (· · · · · · · · · · · · · · · · · · ·		VALES -	Securitized Start		

29	86	1	C	CEDAR	YES	3
31)	96	1	Α	UPIOWA	YES	6
31	97	1	С	MISSOURI	YES	4
32	98	1	А	CEDAR	YES	7

Floodplain sites

- 1. The pollution enclave's shape and position are highly predictable in the alluvial materials near rivers.
- 2. The area underlain by contaminated ground water can be minimized by placing disposal sites adjacent to a river in a groundwater flow regimen where the flow lines are oriented normal to the river.
- 3. Rivers act as boundaries for leachate migration and are areas of discharge for it. Dilution is the principle method of attenuation.
- 4. Depth of penetration of the pollution enclave is minimized in a floodplain position due to the short distance of flow to a discharge point.
- 5. Ground water flow is controlled much more by river stage than by local amounts of precipitation in an alluvial system.

- 6. Enclave size in alluvium reaches a maximum size quickly and then is maintained in a steady-state equilibrium condition.
- 7. Water quality data suggests that contaminated ground water does not significantly affect the surface water into which it discharges.
- 8. The aggregate effect of a number of landfills in floodplain positions within a single river basin must be considered. The cumulative amounts of pollution added should not increase pollution concentrations within the river beyond certain limits.

Upland Sites

- The shape and position of pollution enclaves from upland disposal sites are more difficult to predict due to the natural variability of materials and the great distance to points of ground water discharge.
 The area underlain by contaminated water, therefore, has the potential to be very large in upland sites.
- The distance to points of discharge are greater, and consequently the processes of sorption, decay, and filtration have a greater chance to reduce the concentration of leachate from upland sites before it discharges into surface waters. Upland sites, however, are also regional and local recharge areas for bedrock aquifers.
 The depth of penetration of the pollution enclave may be great and may reach into valuable bedrock aquifers.

- 5. Ground water flow is controlled by the amount of precipitation and the rate of infiltration in upland sites. The range of permeability of the various media present is also important in determining rates and direction of flow.
- 6. Because the available moisture depends on precipitation only, the enclave forms much more slowly than those which develop from disposal sites in floodplains.
- 7. Limited initial data from upland sites indicates that leachate concentrations are lower than those found in the floodplain. This may be due both to topographic position and the relative ages of the sites tested.

 The long term effects of upland sites on water quality are not fully understood at this time.

Status of Iowa Landfills

- The effects of current state regulations favoring the placement of sanitary landfills in upland positions is evident (only 5.4% occur in the floodplain and terrace positions combined).
- Approximately 35% of the permitted landfills have required monitoring programs.
- 3. The percentage of the total number of landfills in the state occurring in any one IGS hazard zone is closely related to the area of that zone. This reflects both population distribution and the cost of transportation as an important economic variable in waste disposal.

- 4. Many sites with non-required monitoring have a potential
 - for special management practices.

RECOMMENDATIONS

 Floodplain sites may be desirable in some cases where bedrock aquifers are vulnerable to contamination and are widely used as sources of water (such as in IGS zone A). An option for the use of these sites should be available in certain situations. 2. Further studies to determine the long term effects of upland disposal sites on the quality of ground and surface water should be started.

PART TWO

THE EFFECT OF UPLAND SANITARY LANDFILL SITES

ON GROUND WATER QUALITY

THE EFFECT OF UPLAND SANITARY LANDFILL SITES ON GROUND WATER QUALITY

REVIEW OF UPLAND SITES

Introduction

The pollution potential of the leachate that is naturally produced in the stabilization of solid wastes depends on many diverse factors including the type and nature of the waste, the rate and amount of infiltration, temperature, and the hydrogeology of the disposal site (Clark, 1975). The nature of the waste depends on local industry and the types of activities performed in the region. In general, the amount of water available for infiltration and the temperature depend on local climate. This leaves the location and therefore the hydrogeology of the site as the only variable which can be chosen to fit optimum conditions within

a local area.

For the most part, regulations governing the operation of disposal sites favor locations on the uplands rather than on the floodplain or terrace. The reason for this is that upland areas generally have a low water table, good surface drainage, and low permeability materials. It is thought that refuse placed in an upland site will remain relatively dry so that the amount of leachate which develops will be minimal. Upland sites are therefore considered more likely to protect the quality of both the ground water and surface water of the surrounding region. Precipitation, however, does occur over upland landfills. Some water infiltrates the surface of the fill, and after passing through the refuse a liquid with high concentrations of both organic and inorganic matter may be formed. Because upland areas are local and regional ground water recharge sites this contaminated water can be very important.

The current analysis of flow patterns (Toth, 1963; LeGrand, 1965; Sendlein and Palmquist, 1975) shows that upland sites have outward flow of both ground and surface water. Because these sites are recharge areas the greatest potential for extensive contamination of bedrock aquifers exists. The long horizontal and vertical distances to zones of discharge allow a much larger and deeper enclave to develop than those of floodplain sites. If the natural attenuation processes are not sufficient to reduce the concentration of the leachate to acceptable levels as it moves through the ground, regional bedrock aquifers may be threatened. For this reason it

50

was decided that more information was needed from landfills in upland positions.

Method of Study

During the spring of 1975 two new landfills in upland sites were instrumented by the Iowa State University Geology Group. One of these landfills was in Scott County and the other one was located in Fayette County. In the summer of 1976 an additional site in Cerro Gordo County was instrumented to monitor ground water quality (Figure 9). All of these sites represent upland areas, but a wide variety of geologic conditions exist. All of the upland sites currently being studied have been operated as sanitary landLYON OSCEOLA DICKINSON EMMET WORTH KOSSUTH WINNEBAGO CERRO GORDO SIOUX O'BRIEN CLAY PALO ALTO HANCOCK CERRO GORDO COUNTY LANDFILL PLYMOUTH FRANKLIN CHEROKEE BUENA VISTA WRIGHT POCAHONTAS HUMBOLDT WEBSTER WOODBURY IDA SAC CALHOUN HAMILTON HARDIN MONONA CRAWFORD MARSHALL CARROLL STORY GREENE BOONE MARRISON JASPER SHELBY POLK AUDUBON SUTHRIE DALLAS POTTAWATTAMIE CASS MARION ADAIR WARREN MADISON MILLS MONTGOMERY ADAMS UNION CLARKE LUCAS FREMONT PAGE TAYLOR WAYNE DECATUR RINGGOLD

Figure 9. LOCATIONS OF CURRENT MONITORING PROGRAMS ON UPLAND SITES



fills since their beginning.

The initial investigation of these landfills has included a drilling program, geophysical investigation, and the collection of water samples for analysis. An observation network of ground water monitor wells was constructed at each site. Well placement was chosen on the basis of the expected shape of the water table inferred from the topography and data obtained from previously existing domestic water-supply wells in the area. At each site wells were placed upflow in an attempt to sample uncontaminated ground water and to establish background quality. Other wells were placed in various directions and at different distances downflow of the site to detect variations in leachate quality. In general, monitor wells were drilled utilizing a hardzog auger, and cased with 1½ inch 0. D. PVC plastic pipe. The lower 3-5 feet was slotted and after the pipe was placed in the ground the screen was packed with gravel. The wells were then sealed from contamination by

52

surface water using bentonite. Other test wells, including domestic water-supply and monitor wells installed by the operators were cased with metal pipe.

The depth of the wells ranged from about 5-45 feet. The variation in depth is the result of trying to intersect water contained in a variety of geologic materials as well as the use of nests of wells. Each well nest normally contains two or three wells seperated horizontally by about 5 feet and terminating at different vertical depths. These well nests help provide data on the vertical as well as horizontal distribution of ground water quality.

Leachate Sampling and Analysis

Each monitor well was sampled approximately once each month, as weather permitted, using a bailer. In the field the static water level, water temperature, and in some cases pH were determined. The samples were then stored on ice in polyethylene bottles during transport to the laboratory where they were refrigerated until the remainder of analyses were completed. Standard wet chemical analyses for a variety of parameters utilized Hach Chemical Company equipment and procedures.

SCOTT COUNTY SITE (SE%, NE%, Sec. 34, T78N, R3E)

The Scott County Landfill is located near the Brady Street exit on Interstate 80 in Davenport, Iowa. It is an upland site in an area of loess-mantled topography (Figures 10& 11), and occupies a position adjacent to a small stream which runs to the east and south of the area (Figure 12). This stream normally flows throughout the year, but during the fall of 1976 it completely dried up due to the extreme lack of precipitation. Several small gullies also border the site.

Currently 17 monitor wells and 2 stream samples are being collected monthly for analysis. Well 8 was the original background well, but preliminary tests indicated high concentrations of some chemical species. Well 9 was then located as a second background well. This well shows a high concentration of Cl⁻ which is undoubtably related to de-icing on the adjacent exit ramp of I-80. Well 10 was drilled as a soil test well to the west of the initial fill area, but it was cased and is used as a sample well. Filling has now progressed to the area surrounding well 10 and will continue until the closure of the site. Future analysis should show the change in quality of the ground water caused by the disposal of refuse. The remainder of the wells are placed in an attempt to intercept leachate migration from the landfill to the adjacent small stream (Table 15).

Both wells 1 and 2 penetrate the refuse disposed in the landfill. Well number 1 has slots open into the garbage and water samples from this well should give an indication of the concentration of the leachate as it leaves the site before any significant attenuation occurs. Well 2 is slotted only into the till beneath the refuse. Water samples from this well indicate the quality of water beneath the site and show the attenuation that is occurring in the underlying till.

Data from this site (Table 16) are still somewhat preliminary and no effort has been made to do a detailed analysis at this time. In general, however, it appears that the quality of water recovered from the shallow wells (those with a "b" suffix) is lower than the water obtained from the adjacent deeper wells. Most of the shallow wells terminate within the loess above the loess-till interface. The presence of leachate with higher ion concentrations in the shallow wells may be due to a channelization of ground water flow in the loess above the less permeable till. It may also be

an indication of the increased attenuation ability offered by the till in which the deeper wells are completed.

10. SCOTT COUNTY LANDFILL FIGURE Generalized North-South Cross Section



200 0 200 SCALE IN FEET

400 VERTICAL EXAGGERATION:5

FIGURE 11. SCOTT COUNTY LANDFILL Generalized West-East Cross Section





WELL 8.

57

F

150 300 VERTICAL EXAGGERATION: 3.75

FIGURE 12. SCOTT COUNTY LANDFILL

Monitor Well Location Map

-80

STREAM

9.

-80

N



TABLE 15.

SCOTT COUNTY LANDFILL Guide to Monitor Wells

Well 1 - Total Depth 38½ feet. Screen open into filled material. Well 2 - Total Depth 60 feet. Screen open into till beneath fill. Well 4a - Total Depth 30 feet. Screen open in loess. Well 4b - Total Depth 20 feet. Screen open in loess. Well 5a - Total Depth 38½ feet. Screen open into top of till. Well 5b - Total Depth 28 feet. Screen open into loess. Screen open 2 feet into till. Well 6a - Total Depth 30 feet. Well 6b - Total Depth 20 feet. Screen open into loess. Screen open into top of till. Well 7 - Total Depth 20 feet. Well 7a - Total Depth 40 feet. Screen open into loess. Well 8 - Total Depth 21 feet. Screen open into 4 feet of till. Screen open into 2 feet of till. Well 9 - Total Depth 21 feet.

Well 10 - Former Soil Boring, T. D. 50 feet. Screen open to till.
Well 11 - Total Depth 34 feet. Screen open into 4 feet of till.
Well 12a - Total Depth 25 feet. Screen open into till.
Well 12b - Total Depth 16 feet. Screen open into till.

TABLE 16. Water Quality at Scott County Landfill

.

WELL	PARAM	LO	ΗI	AVE
1	PH '	6.00	7 5 6	
	SPECCOND	200.00	1.55	6.89
	DO	0.00	2250.00	1418.00
	ALK	240.00	1120.00	7.68
	CAHRD	135.00	1795 00	560.93
	MGHRD	22.00	1635.00	532.57
	CA/MG	0.22	47.27	454.64
	FÉ	0.04	16.00	469
	SC4	0.00	58.00	3.25
	CL	20.00	1220.00	547 50
	NO3	0.31	3.90	1 343.50
_	MN	0.20	7.00	3.06
2	PH	6.67	8.54	7.30
	SPECCOND	180.00	455.00	379.71
C	DO	7.36	34.20	15-81
	ALK	215.00	536.0C	420.41
	TTLHRD	180.00	2450.00	500.94
	CAHRD	105.00	1710.00	325.88
	MGHRD	50.00	740.00	175.06
	CAMG			2.03
	FE	0.02	4.10	0.99
	S04	0.05	22.50	5.58
	CL	15.00	50.00	22.68
	NO3	0.00	1.75	0.29
4A	РН	6.73	8.58	7.65
	SPECCOND	85.00	340.00	244.41
	DO	10.60	39.60	20.14
	ALK	120.00	450.00	337.56
	TILHRD	100.00	450.0C	304.60
	CAHRD	30.00	305.00	193.06
	MGHRD	60.00	177.00	111.56
	CAYMG	0.43	3.59	1.77
	504	0.02	0.72	. 0.16
	504	0.00	32.50	4.92
	NOR	4.00	20.50	7.14
	MN	0.09	1.55	0.52
4B	PH	0.00	0.65	0.28
	SPECCOND	100.00	8.49	7.56
	DO	11.50	390.00	278.24
	ALK	105.00	25.00	19.21
	TTLHRD	86.00	684 00	305.22
	CAHRD	55.00	760.00	360.67
	MGHRD	25.00	414-00	248.39
	CA/MG	0.39	9.40	112.28
	FE	0.01	0.40	2.98
	S04	16.00	72.00	40.44
100 000	CL	5.00	17.50	9.44
	NO3	0.12	1.10	0.51
	MN	0.00	1.30	0.30
5A	PH	6.87	8.48	7.65
	SPECCOND	150.00	450.00	319.59
	ALK	148.00	483.00	382.11
	TTLHRD	168.00	559.00	412.00

×.

WELL	PARAM	LO	ΗI	AVE
	CAHRD	35.00	355.00	266.94
	MGHRD	71.00	232.00	145.06
	CAMG	0.26	3.82	2.00
	FE	0.00	1.38	0.18
181	504	10.00	57.00	33.13
	CL	6.50	22.00	13.03
	NO3	0.10	2.10	0.53
	MN	0.10	3.00	0.82
	PH	6.48	8.44	7.12
50	SPECCOND	195.00	775.00	508.71
	DO	8.70	25.00	16.70
	ALK	156.00	1018.00	654.06
	TTLHRD	236.00	1050.00	712.06
	CAHRD	46.00	765.00	462.67
	MGHRD	105.00	400.00	249.39
	CAMG	0.24	4.10	2.00
	FE	0.05	1.88	0.48
	S04	47.50	130.00	90.08
	CL	5.00	9.50	7.47
	NC3	0.05	0.85	C . 28
	MN	0.00	4.65	1.12
6A	PH	6.95	8.40	7.55
	SPECCOND	120.00	00.08E	283.53
	DO	11.04	36.80	17.27
	ALK	170.00	455.00	348.56
	TTLHRD	140.00	505.00	336.89
	CAHRD	60.00	345.00	213.72
	MGHRD	73.00	206.00	123.17
	CAMG	0.47	4.26	1.86
	FE	0.01.	0.65	. 0.13
	SC4	0.00	36.50	8.49
	CL	5.00	11.00	6.86
	NO3	0.06	1.13	0.46
_	MN	. 0.00	2.80	0.99
6B	PH	6.68	8.45	7.31
	SPECCOND	205.00	530.00	379.41
	DO	9.20	39.40	17.72
	ALK	162.00	640.00	496.33
	TTLHRD	275.00	785.00	543.28
	CAHRD	125.00	497.00	349.28
	MGHED	95.00	342.00	194.00
	CAMG	0.66	4.75	1.98
	FE	C.C.1	1.00.00	0.25
	S04	11.50	100.00	C1 • 1C
		4.50	1.07	0.30
	NU3	0.00	0.75	0.25
77	DU	7.23	8.04	7.92
TA	SPECCOND	135-00	365.00	295.83
	DO	7.40	19.78	10.30
	ALK	153.00	450.00	342.57
	TTLHPD	140-00	432.00	332.14
	CAHRD	50.00	280.00	203.72
	MGHRD	70.00	161.00	128.43
		The second se	Contraction resources (E.S.)	The state of the s

WELL	PARAM	LO	۲H	AVE
Ser. Ser	CAMG	0.56	2.25	
	FE	0.01	1 2022	1.56
	504	1.00	1.24	0.24
	CL	5.00	28.40	10.41
	NOR	5.00	18.50	5.64
	MN	0.10	0.62	0.31
7	PH	0.10	1.20	C.40
1	SPECCOND	100 50	8.52	7.67
	SPECCOND	120.00	352.00	244.00
		10.10	32.60	17.62
	TTUNED	150.00	433.00	320.78
	TILHRU	140.00	517.00	313.83
	CAMRD	50.00	350.00	201.94
	MGHRD	26.00	180.00	. 111.89
	CAMG	0.52	9.81	2.16
	FE	0.0.1	0.87	0.13
	S04	0.00	12.40	6.86
	CL	5.00	13.50	8.08
	N03	2.10	1.22	0.50
25 A	MN	0.02	0.30	0.13
7B	PH	6.7.7	8.40	7.72
	SPECCOND	205.00	365.00	287.14
	DO	10.60	24.38	15.07
	ALK	300.00	420.00	371.88
	TTLHRD	325.00	421.00	378.25
	CAHRD	195.00	286.00	246.87
	MGHRD	95.00	163.00	131.38
	CAMG	1.44	2.55	1.93
	FE	0.02	0.46	0.13
	S04	2.00	33.00	25.44
	CL '	5.00	10.00	8.56
	NC3	0.12	0.60	0.27
	MN	0.00	0.25	0.06
8	PH	6.83	8.58	7.55
	SPECCEND	42.00	375.00	260 .83
	DC	11.50	54.60	10.33
	ALK	62.00	470.00	324 30
	TTLHRD	55.00	480-00	345.29
	CAHRD	30.00	365.00	530 00
· File in	MGHRD	25.00	180.00	106 20
	CAMG	0.55	5.90	11.0.20
	FE	0.00	5.69	2.47
32 k 1.	504	0.00	56 0C	0.09
11	CI	5.00	50.00	29.08
	NOR	0.15	10.00	10.36
	MN	0.00	1.50	0.65
9	PH	7.00	<u></u>	5.12
	SPECCOND	160 00	. 0.00	1.68
	DO	100.00	410.00	298.85
	ALK	10.00	4: • 40	19.20
	TIHPO	124.00	365.00	. 267.93
	CAHED	107.00	472.00	330.57
	NGHED	107.00	337.00	228.64
	CAME	25.00	184.00	11.93
	CAMIG	1.11	9.57	3.00

.

$\begin{array}{c c c c c c c c c c c c c c c c c c c $	WELL	PARAM	LO	HI	AVE
$\begin{array}{c c c c c c c c c c c c c c c c c c c $					
SC4 13.00 44.07 31.69 CL 12.50 75.07 47.08 N03 0.77 1.60 7.52 10 PH 7.11 8.55 7.75 SPECCCND 140.00 350.07 268.00 00 14.30 25.40 19.44 ALK 180.00 828.00 325.06 THHRD 180.00 390.00 325.06 MCHRD 20.00 135.00 44.25 CAMG 1.00 19.50 3.67 FE 0.00 3.04 6.24 SC4 16.00 56.00 3.67 FE 0.00 3.04 6.24 SC4 16.00 15.00 44.25 CAMG 1.00 19.50 3.67 FE 0.00 3.04 6.20 CAMG 1.00 17.22 6.35 MC 0.00 8.70 7.24 SPECCOND 320.00 6		FF	0.03	2.60	0.71
CL 12.50 77.02 14.08 N03 0.07 1.60 7.408 N03 0.07 1.60 7.50 10 PH 7.11 8.55 7.75 10 SPECCEND 140.00 350.00 268.00 DO 14.30 25.40 19.44 ALK 180.00 828.20 325.06 TTLHRD 180.00 390.00 224.63 MGHED 20.00 3.67 44.25 CAMG 1.00 19.50 3.67 FE 0.00 3.0.74 6.24 SC4 16.00 56.00 37.28 CL 4.00 16.50 8.70 NC3 0.05 0.72 6.35 MN 0.00 2.00 0.05 ALK 290.00 875.00 21.00 NC3 0.05 0.72 15.50 ALK 290.00 844.00 79.00 ALK 290.		504	13.00	44.00	31.69
NO3 0.07 1.60 0.400 MN 0.00 1.60 0.400 10 PH 7.11 8.55 7.75 SPECCOND 140.00 350.00 266.00 DO 144.30 25.40 19.44 ALK 180.00 828.00 325.06 TLHRD 180.00 416.00 336.88 CAHRD 90.00 390.00 242.63 MC4 1.00 19.50 3.67 FE 0.000 3.04 C.242.63 MC4 16.00 56.00 3.67 FE 0.000 3.04 C.242.63 MC4 16.00 16.50 8.70 NC3 0.05 7.72 6.35 MN 0.00 2.00 2.00 D0 11.50 17.02 15.69 ALK 290.00 875.00 216.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 <td></td> <td>CL</td> <td>12.50</td> <td>75.00</td> <td>47.08</td>		CL	12.50	75.00	47.08
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	· · · · · · · · · · · · · · · · · · ·	NOR	0.07	1.60	0.50
ID PH 7.11 2.55 7.75 SPECCEND 140.00 350.00 264.00 DO 14.30 25.40 194.4 ALK 180.00 628.00 325.06 TTLHAD 180.00 390.00 242.63 MGHRD 20.00 3.00 244.25 CAMS 1.00 19.50 3.67 FE 0.00 3.04 6.24 SC4 16.00 56.00 37.28 CL 4.00 16.50 8.70 NC3 0.05 9.72 6.35 MN 0.06 2.00 0.05 DO 11.50 17.02 15.69 ALK 290.00 875.00 232.00 CAHRD 133.00 644.00 719.00 CAHRD 133.00 644.00 719.00 CAHRD 133.00 105.00 719.00 CAHRD 133.00 130.00 105.00 CAHRD <		MN	0.00	C.80	0.30
10 SPECCUND 140.00 350.00 266.00 D0 144.30 25.40 19.444 ALK 180.00 828.00 325.60 TLHRD 180.00 410.00 336.88 CAHRD 90.00 390.00 242.63 MGHRD 20.00 3.64 C.24 SCA 16.00 56.00 3.67 FE 0.00 3.64 C.24 SD4 16.00 56.00 8.70 NC3 0.005 0.72 6.35 MN 3.005 0.772 6.35 MN 3.005 17.702 15.60 ALK 290.00 665.00 523.00 D0 11.50 17.02 15.60 ALK 290.00 664.00 719.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 0 64.00 224.63 CAMG 0.45 2.16 1.55 <td< td=""><td>10</td><td>PH</td><td>7.11</td><td>9,55</td><td>7.75</td></td<>	10	PH	7.11	9,55	7.75
DD 14.30 25.40 19.44 ALK 180.00 828.90 325.06 TTLHRD 180.00 416.00 336.90 CAHRD 90.00 390.00 242.63 MGHRD 20.00 135.00 44.25 CAMG 1.00 19.50 3.67 FE 0.00 3.04 C.24 SC4 16.00 56.00 37.28 CL 4.00 16.50 6.70 ND3 0.005 0.72 6.35 MN 0.002 2.00 665.00 523.00 DD 11.50 17.02 15.69 ALK 290.00 675.00 216.10 ALK 290.00 675.00 224.83 CAMRD 232.00 664.00 434.17 MGHRD 232.00 675.00 284.83 CAMG 0.445 2.16 1.55 FE 0.02 0.80 12.50 105.03 <t< td=""><td>TO</td><td>SPECCEND</td><td>140.00</td><td>350.00</td><td>268.00</td></t<>	TO	SPECCEND	140.00	350.00	268.00
ALK 180.00 828.00 325.06 TTLHAD 180.00 390.00 242.63 CAHRD 90.00 390.00 242.63 MGHRD 20.00 135.00 44.25 CAMG 1.00 19.50 3.67 FE 0.00 3.04 C.24 SC4 16.00 56.00 37.28 CL 4.00 16.50 8.70 NC3 0.05 0.72 0.35 MN 0.00 2.00 2.00 DO 11.50 17.02 15.69 ALK 290.00 875.00 617.50 TTLHRD 581.00 719.00 CAHRD 133.00 644.00 131.00 CAMRD 232.00 364.00 224.63 CAMRD 133.00 644.00 74.63 MGHRD 232.00 364.00 224.63 CAMRD 133.00 644.00 224.63 CAMRD 133.00 644.0		DO	14.30	25.40	19.44
TTLHRD 180.00 410.00 336.88 CAHRD 90.00 390.00 242.63 MGHRD 20.00 135.00 44.26 CAMG 1.00 19.50 3.67 FE 0.00 3.04 0.24 SC4 16.00 56.00 3.74 CL 4.00 16.50 8.70 ND3 0.05 0.72 0.35 MN 0.06 2.00 9.00 DO 11.50 17.02 15.69 ALK 290.00 875.00 617.50 TTLHRD 581.30 961.00 719.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 264.00 244.83 CAMG 0.45 2.16 1.55 FE 0.02 0.80 12.50 100 CL 8.00 12.50 100 105.00 CL 8.00 12.50 100 105.00		ALK	180.00	828.00	325.06
CAHRD 90.00 390.00 242.63 MGHRD 20.00 135.00 44.25 CAMG 1.00 19.50 3.67 FE 0.00 3.44 C.24 SC4 16.00 56.00 37.28 CL 4.00 16.50 8.70 NC3 0.05 0.722 6.35 MN 0.00 2.00 0.08 DO 1150 17.02 15.69 ALK 290.00 875.00 617.50 TTL FRD 581.00 718.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 264.100 719.00 CAHRD 133.00 644.00 244.63 CAMRD 133.00 644.00 244.17 MGHRD 232.00 105.00 105.00 CL 8.00 12.50 10.03 CL 8.00 12.450 10.03 MN 0.10 2.60 <		TTI HRD	180.00	410.00	336.88
MGHRD 20.00 135.00 44.25 CAMG 1.00 19.50 3.67 FE 0.00 3.04 C.24 SCA 160 56.00 37.28 CL 4.00 16.50 8.70 NC3 0.05 7.72 C.35 MN 0.00 26.00 665.00 522.00 DO 11.50 17.02 15.69 ALK 290.00 875.00 617.50 TTL.HRD 581.30 961.00 719.00 CAMRD 133.00 644.00 434.17 MGHRD 232.00 364.00 224.03 SD4 81.00 130.00 105.00 CL 8.00 12.50 11.08 ND3 0.13 1.23 C.43 SPECCUND 69.00 42500 279.50 TLHRD 50.00 394.00 279.50 TLHRD 90.00 688.01 294.50 CAMRG		CAHRD	90.00	290.00	242.63
CAMAG 1.00 19.50 3.67 FE 0.00 3.44 C.24 SC4 16.00 56.00 37.28 CL 4.00 16.50 8.70 NC3 0.05 1.72 6.35 MN 0.00 2.00 0.08 MN 0.00 2.00 0.55 DD 11.50 17.02 15.69 ALK 290.00 875.00 522.00 DD 11.50 17.02 15.69 ALK 290.00 875.00 617.50 TTL HRD 581.00 719.00 719.00 CAMAG 0.445 2.16 1.55 FE 0.02 0.60 719.00 CAMAG 0.45 2.16 1.55 FE 0.02 0.60 6.24 S04 81.00 130.00 105.00 CL 8.00 12.50 12.50 DO 4.60 17.48 12.33		MGHRD	20.00	135.00	44.25
FE 0.00 3.94 C.24 SG4 16.0C 56.0C 37.28 CL 4.00 16.50 8.70 NC3 0.05 0.72 G.35 MN 0.060 2.00 0.08 11 PH 6.49 8.10 7.24 SPECCOND 326.00 665.00 523.00 CD 1150 17.02 15.69 ALK 290.00 875.00 644.70 GAMRD 133.00 644.00 434.17 MGHRD 232.00 354.00 284.83 CAMRG 0.45 2.16 1.55 FE 0.02 0.80 0.24 SD4 81.00 12.50 105.00 CL 8.00 12.50 10.08 ND3 0.13 1.23 C.43 MN 0.10 2.60 29.50 TTLHRD 50.00 686.00 294.50 CL 8.50 293.		CAMG	1.00	19.50	3.67
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$		FF	0.00	3.04	C.24
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	-	504	16.00	56.00	37.28
$\begin{array}{c cccc} & & & & & & & & & & & & & & & & & $		CL	4.00	16.50	8.70
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$		NC3	0.05	0.72	0.35
11 PH 6.45 8.10 7.24 SPECCOND 326.00 665.00 523.00 CD 11.50 17.02 15.69 ALK 290.00 875.00 617.50 TTL HRD 581.30 961.00 719.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 354.00 284.83 CAWG 0.45 2.16 1.55 FE 0.02 0.80 0.200 SD4 81.00 12.50 100.00 CL 8.00 12.50 10.08 ND3 0.13 1.23 C.443 MN 0.10 2.60 7.94 SPECCUND 69.00 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTL+RD 90.00 4.60 17.48 12.33 ALK 85.00 395.00 279.50 TTL+RD 90.00 6.92 2.01		MN	0.00	2.00	0.08
II SPECCOND 320.00 665.00 £23.00 DD 11.50 17.02 15.69 ALK 290.00 875.00 617.00 TTL HRD 581.30 961.00 719.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 354.00 284.83 CAMKG 0.455 2.16 1.55 FE 0.02 0.80 0.224 S04 81.00 130.00 105.00 CL 8.00 12.50 10.08 ND3 0.13 1.23 C.433 MN C.10 2.60 C.92 I2A PH 6.92 8.73 7.94 SPECCUND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTL FRD 96.00 688.00 294.50 CAHRD 42.00 395.00 115.00	11	PH	6.49	8.10	7.24
DD 11.50 17.02 15.69 ALK 290.00 875.00 617.50 TTLHRD 581.30 961.00 719.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 354.00 224.83 CAMG 0.45 2.16 1.55 FE 0.02 0.80 0.24 S04 81.00 130.00 105.00 CL 8.00 12.50 10.08 N03 0.13 1.23 0.43 MN 0.10 2.80 0.92 12A PH 6.92 8.73 7.94 SPECCUND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLHRD 96.00 688.00 294.50 CAHRD 42.00 393.00 115.00 CAHRD 42.00 293.00 115.00 CAMG <	11	SPECCOND	320.00	665.00	523.00
ALK 290.00 875.00 617.50 TTL HRD 581.30 961.00 719.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 354.00 284.83 CAWG 0.45 2.16 1.55 FE 0.02 0.80 0.24 S04 81.00 130.00 105.00 CL 8.00 12.50 10.08 ND3 0.13 1.23 C.43 NN 0.10 2.80 7.92 12A PH 6.92 8.73 7.94 SPECCUND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLERD 96.000 688.00 294.50 CAHRD 42.00 293.00 115.00 CAMG 0.88 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.600		CO	11.50	17.02	15.69
TTL HRD 581.00 961.00 719.00 CAHRD 133.00 644.00 434.17 MGHRD 232.00 354.00 224.83 CAMG 0.45 2.16 1.55 FE 0.02 0.80 C.24 SD4 81.00 130.00 105.00 CL 8.00 12.50 10.08 ND3 0.13 1.23 C.43 MN 0.10 2.80 C.92 12A PH 6.92 8.73 7.94 SPECCUND 69.00 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLFRD 90.00 6.88.00 294.50 CAHRD 42.00 395.00 115.00 CAHRD 42.00 395.00 115.00 CAHRD 42.00 395.00 115.00 CAHRD 46.00 79.50 13.10 CL 5.50 20.00 9.13		ALK	290.00	875.00	617.50
CAHRD 133.00 644.00 434.17 MGHRD 232.00 354.00 284.83 CAWG 0.45 2.16 1.55 FE 0.02 0.86 0.24 SD4 81.00 130.00 105.00 CL 8.00 12.50 10.08 ND3 0.13 1.23 C.43 MN 0.10 2.80 7.94 SPECCUND 69.00 425.00 219.25 DO 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLERD 90.00 688.00 294.50 CAHRD 42.00 395.00 115.00 CAHRD 42.00 395.00 115.00 CAMRG 0.88 2.68 1.60 FE 0.00 0.88 2.00 9.13 NC3 0.10 0.46 0.21 500 S04 0.00 0.90 9.13 100		TTLHRD	581.00	961.00	719.00
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		CAHRD	133.00	644.00	434.17
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		MGHRD	232.00	354.00	284.83
FE 0.02 0.86 0.24 SD4 81.00 130.00 105.00 CL 8.00 12.5C 10.08 ND3 0.13 1.23 0.43 MN 0.10 2.80 0.92 12A PH 6.92 8.73 7.94 SPECCUND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLERD 90.00 688.00 294.50 CAHRD 42.00 395.00 115.00 CAMG 0.888 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.000 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 0.46 0.21 SD4 0.00 2.65 0.16 MN 0.000 2.60 9.13 NC3 0.10 0.28		CAMG	0.45	2.16	1.55
S04 81.00 130.00 105.00 CL 8.00 12.5C 10.08 N03 0.13 1.23 0.43 MN 0.10 2.80 0.92 12A PH 6.92 8.73 7.94 SPECCUND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLFRD 90.00 688.00 294.50 CAHRD 42.00 395.00 115.00 CAMRD 42.00 395.00 115.00 CAMRD 42.00 395.00 115.00 CAMRD 42.00 395.00 115.00 CAMRD 0.88 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 0.46 0.21 SD4 0.00		FE	0.02	0.80	C.24
CL 8.00 12.5C 1C.08 NO3 0.13 1.23 C.43 MN 0.10 2.80 C.92 12A PH 6.92 8.73 7.94 SPECCOND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLFRD 90.00 688.00 294.50 CAHRD 42.00 395.00 115.00 CAMG 0.88 2.68 1.60 FE C.01 0.46 0.21 S04 0.000 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 0.46 0.21 S04 0.00 2.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 D0 6.90 20.24 13.81 ALK 63.00 <td></td> <td>S04</td> <td>81.00</td> <td>130.00</td> <td>105.00</td>		S04	81.00	130.00	105.00
NO3 0.13 1.23 C.43 MN 0.10 2.80 C.92 12A PH 6.92 8.73 7.94 SPECCOND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLHRD 96.00 688.00 294.50 CAHRD 42.00 395.00 115.00 CAHRD 42.00 293.00 115.00 CAMG 0.88 2.68 1.60 FE C.01 0.46 0.21 SU4 0.00 79.50 13.10 CL 5.50 20.30 9.13 NC3 0.10 C.45 0.45 MN 0.00 2.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00		CL	8.00	12.50	10.08
NN 0.10 2.80 0.92 12A PH 6.92 8.73 7.94 SPECCUND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTL HRD 90.00 688.00 294.50 CAHRD 42.00 395.00 115.00 CAHRD 42.00 293.00 115.00 CAMG 0.888 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 0.46 0.21 SD4 0.00 2.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 365.00 250.00 D0 6.90 20.24 13.81 ALK 63.00 378.00 302.13 CAHRD		NO 3	0.13	1.23	C.43
12A PH 6.92 8.73 7.94 SPECCUND 69.00 425.00 219.25 D0 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTL FRD 90.00 688.00 294.50 CAHRD 42.00 395.00 179.50 MGHRD 48.00 293.00 115.00 CAMG 0.88 2.68 1.60 FE 0.01 0.46 0.21 S04 0.000 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 0.46 0.21 S04 0.00 2.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 D0 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTL HRD 65.00 428.00 302.13 CAHRD <td></td> <td>MN</td> <td>0.10</td> <td>2.80</td> <td>r.92</td>		MN	0.10	2.80	r.92
SPECCUND 69.0C 425.00 219.25 DQ 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLHRD 90.00 688.00 294.50 CAHRD 42.00 395.00 179.50 MGHRD 48.00 293.00 115.00 CAMG 0.88 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.000 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 C.28 C.16 MN 0.000 2.655 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 202.13 CAHRD 30.00 <td>12A</td> <td>PH</td> <td>6.92</td> <td>8.73</td> <td>7.94</td>	12A	PH	6.92	8.73	7.94
DO 4.60 17.48 12.33 ALK 85.00 394.00 279.50 TTLFRD 90.00 688.00 294.50 CAHRD 42.00 395.00 179.50 MGHRD 48.00 293.00 115.00 CAMG 0.88 2.68 1.60 FE 0.01 0.46 0.21 SU4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 0.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		SPECCUND	69.00	425.00	219.25
ALK 85.00 394.00 279.50 TTL FRD 90.00 688.00 294.50 CAHRD 42.00 395.00 179.50 MGHRD 48.00 293.00 115.00 CAMG 0.888 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 C.28 C.16 MN 0.00 0.655 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 D0 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTL HRD 65.00 428.00 302.13 CAHRD 30.00 20.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.56 C.29 <td></td> <td>DO</td> <td>4.60</td> <td>17.48</td> <td>12.33</td>		DO	4.60	17.48	12.33
TTLLHRD 90.00 688.00 294.50 CAHRD 42.00 395.00 179.50 MGHRD 48.00 293.00 115.00 CAMG 0.88 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 0.465 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 20.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		ALK	85.00	394.00	279.50
CAHRD 42.00 395.00 179.50 MGHRD 48.00 293.00 115.00 CAMG 0.888 2.68 1.60 FE 0.01 0.46 0.21 SU4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 C.28 C.16 MN 0.00 0.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.224 13.81 ALK 63.00 378.00 250.00 DO 6.90 20.224 13.81 ALK 63.00 378.00 302.13 CAHRD 30.00 307.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		TTLFRD	90.00	688.00	294.50
MGHRD 48.00 293.00 115.00 CAMG 0.88 2.68 1.60 FE 0.01 0.46 0.21 SU4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 C.28 C.16 MN 0.00 0.655 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 D0 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		CAHRD	42.00	395.00	179.50
CAMG 0.88 2.68 1.60 FE 0.01 0.46 0.21 SD4 0.00 79.50 13.10 CL 5.50 20.00 9.13 NC3 0.10 C.28 0.16 MN 0.00 0.655 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		MGHRD	48.00	293.00	115.00
FE C • 0 1 0 • 46 G • 21 SU4 C • 0 0 79 • 5 C 13 • 10 CL 5 • 50 20 • 30 9 • 13 NC3 C • 1 C C • 28 C • 16 MN 0 • 0 C C • 65 0 • 15 12B PH 7 • 38 8 • 67 8 • 00 SPECCOND 67 • 00 385 • 00 25 0 • 00 DO 6 • 90 20 • 24 13 • 81 ALK 63 • 00 378 • C 0 273 • 38 TTL HRD 65 • 00 428 • 00 302 • 13 CAHRD 30 • 00 307 • C C 179 • 38 MGH FD 30 • 00 200 • 00 122 • 75 CAMG 0 • 26 2 • 54 1 • 48 FE 0 • 01 1 • 96 C • 29		CAMG	0.88	2.68	1.60
S04 C.CC 79.5C 13.10 CL 5.50 20.00 9.13 NC3 C.1C C.28 C.16 MN 0.00 C.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		FE	C.01	0.46	6.21
CL 5.50 20.30 9.13 NC3 0.10 0.28 0.16 MN 0.00 0.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 D0 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		504	0.00	79.50	13.10
NU3 0.10 0.25 0.16 MN 0.00 0.65 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 D0 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		CL	5.50	20.00	9.1.5
MN 0.00 0.00 0.05 0.15 12B PH 7.38 8.67 8.00 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 179.38 MGHRD 30.00 200.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		NU3	0.00	0.45	0.15
12B PF 7.56 0.67 0.67 SPECCOND 67.00 385.00 250.00 DO 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 179.38 MGHRD 30.00 200.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29	100	DH	7 38	8.67	8.00
DD 6.90 20.24 13.81 ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 179.38 MGHRD 30.00 200.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.56 0.29	TZB	SPECCOND	67.00	385.00	250.00
ALK 63.00 378.00 273.38 TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 179.38 MGHRD 30.00 200.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		DO	6.90	20.20	17.81
TTLHRD 65.00 428.00 302.13 CAHRD 30.00 307.00 179.38 MGHRD 30.00 200.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.56 0.29		ALK	63.00	378.00	273.38
CAHRD 30.00 307.00 179.38 MGHRD 30.00 200.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.96 0.29		TTI HED	65.00	428.00	302-13
MGHED 30.00 200.00 122.75 CAMG 0.26 2.54 1.48 FE 0.01 1.56 0.29		CAHED	30.00	307.00	179.38
CAMG 0.26 2.54 1.48 FE 0.01 1.56 C.29		MGHED	30.00	200.00	122.75
FE 0.01 1.56 C.29		CAME	0.26	2.54	1.48
		FE	0.01	1.56	C.29

3.

WELL	PARAM	LO	HI	AVE
	504	10.00	63.50	74.60
	CL	7.50	15.50	24.09
	NC3	0.02	1.15	10.00
	MN	0.00	2.00	0.75
S1	PH	7.25	8.53	7.09
DT.	SPECCOND	75.00	400.00	204 07
	DO	8.70	32.00	10.07
	ALK	100.00	318.00	218.67
	TTLHRD	170.00	413.00	310.20
1 4	CAHRD	107.00	362.00	522.17
	MGHRD	45.00	134.00	88.07
1 4	CA/MG	1.26	7.11	2 90
	FE	0.00	0.14	0.00
	SC4	21:50	72.00	47 77
	CL	16.50	182.50	4/03/
	NO3	0.02	43.50	39.01
	MN	0.00	0.10	3.40
.00	PH	7.38	8.75	8 06
04	SPECCOND	155.00	395.00	280.20
	DO	0.20	58.60	21.78
	ALK	80.00	320.00	200.13
	TTIHED	155.00	400.00	202.80
	CAHRD	155.00	759 00	154.27
	MGHED	32.00	175 00	C9 57
	CNNG	0.66	11 10	2.52
	FE	0.00	11.10	C C P
	504	19.00	60.00	45.07
	CL	13.00	114 00	52 67
	ND3	13.50	114.00	1 64
	105	0.04	42.0111	4.04



FAYETTE COUNTY SITE (NW1. Sec. 21, T93N, R8W)

The Fayette County Landfill is located northwest of Fayette, Iowa on property owned by the County Farm. The site is an upland area of loamy soils developed on glacial till (Figures 13 & 14). A small stream runs approximately one-half mile west of the landfill and is the local sink to which leachate migration is expected (Figure 15).

Currently 30 wells are available for water sampling. Wells 4-15 are domestic water-supply wells that were in operation before use of the site for waste disposal began. The remainder of the wells were placed for the current ground water monitoring program. Samples from the wells and a surface water sample from the stream are collected approximately once each month for analysis (Table 17).

The investigation at this site is complicated by the presence

of animal feedlots near many of the domestic supply wells. The entire area surrounding the disposal site is being used for agricultural crop production making the presence of fertilizer and pesticide residuals possible. The sewage treatment lagoon for the Fayette County Home is also located on the southwest of the landfill in the general direction that leachate is expected to migrate toward the stream.

Data from the site are still somewhat preliminary (Table 18). The pattern of shallow wells having higher leachate concentrations than nearby deep wells is not as well expressed at this site as at Scott County. Well nests 2 and 3 do show this general trend, however. This may be an indication of the movement of leachate in the general direction of these well nests. The well nest down gradient from the sewage lagoon (21) has lower concentrations of dissolved oxygen than the rest of the wells. Other parameters, however, are much like those for other wells in the vicinity. It should be noted that well nests 21, 22, and 23 were placed during November of 1976 and the short period of time since emplacement and the general low level of precipitation may not have allowed these wells to fully stabilize at this time.

FIGURE 13.

FAYETTE COUNTY LANDFILL

Generalized North-South Cross Section



FIGURE 14.

FAYETTE COUNTY LANDFILL



Generalized West-East Cross Section



TABLE 17.

FAYETTE COUNTY LANDFILL

Guide to Monitor Wells

- Well 2a Total Depth 38% feet. Screen open into till.
- Well 2b Total Depth 45 feet. Screen open into 3 feet of limestone.
- Well 2c Total Depth 24 feet. Screen open to sand.
- Well 2d Total Depth 15 feet. Screen open to sand.
- Well 3b Total Depth 16 feet. Screen open into till.
- Well 3c Total Depth 4½ feet. Screen open into sand.
- Well 16 Total Depth 291/2 feet. Screen open into till.
- Well 17 Total Depth 18 feet. Screen open into till.
- Well 18 Total Depth 19 feet. Screen open into till.
- Well 19 Total Depth 16% feet. Screen open into sand.
- Well 20 Total Depth 17½ feet. Screen open into till.
- Well 21a Total Depth 48 feet. Screen open into top of limestone
 - and till.

Well 21b - Total Depth 8 feet. Screen open into sand. Well 22a - Total Depth 44 feet. Screen open into silty sand. Well 22b - Total Depth 17 feet. Screen open into silty sand. Well 23a - Total Depth 48 feet. Screen open into till. Well 23b - Total Depth 38 feet. Screen open into till.
TABLE 18. Water Quality at Fayette County Landfill

WELL	PARAM	LO	HI	AVE
1	PH	7.34	8.88	8.47
	SPECCOND	81.00	875.00	182.75
	00	12.88	25.80	17.37
	ALK	80.00	240.00	107.25
	TTLHRD	89.00	168.00	118.83
	CAHRD	35.00	110.00	56.75
	MGHPD	49.00	77.00	62.08
	CANG	0.54	1.90	0.03
	FE	0.02	0.77	0.23
	S04	10.80	37.00	19.50
	CL	10.00	22.50	14.00
	NO3	0.00	0.35	0.18
2A	PH	7.16	8.58	7.92
	SPECCOND	125.00	300.00	227.22
	DO	13.40	24.60	17.68
	ALK	40.00	250.00	179.33
	TTLHRD	121.00	335.00	231.22
	CAHRD	85.00	239.00	157.13
	MGHPD	36.00	120.00	72.25
	CAMG	0.83	3.56	2.44
	FE	0.11	2.00	0.65
	SC4	00.0	125.00	37.75
	CL	21.00	50.00	32.61
	N03	0.02	1.50	0.46
2B	РН	7.43	9.12	8.47
	SPECCOND	62.00	160.00	93.80
	DO	11.00	38.00	18.26
	ALK	0.47	166.00	77.70
	TTLHRD	45.00	127.00	76.50
	CAHRD	20.00	100.00	44.50
	MGHPD	16.00	56.00	32.00
	CAMG	0.55	3.70	1.57
	FE	2.02	C.45	0.10
	504	1.00	28.00	9.04
	CL	12.00	55.00	23.15
	ND.3	0.08	0.28	0.17
2C	Н	6.99	8.54	7.65
	SPECCOND	75.00	245.00	150.27
		13.30	41.00	18.33
	ALK	64.00	195.00	153.07
	CAHED	70.00	330.00	168.00
	MCHED	39.00	160.00	114.55
	CAME	10.00	129.00	33.45
	EE	0.02	1 70	2.11
	504	0.05	105.00	24 01
	CL	7.50	25.50	15.00
	NOR	0.01	2.71	0.49
	PH	6.99	8.54	7.65
	SPECCOND	75.00	245.00	150.27
	DO	13.30	41.00	18.33
	ALK	64.00	195.00	153.00
	TTLHRD	102.00	336.00	168.00
				and the second sec

 \mathbf{x}

- F.

	WELL	PARAM	LO	HI	AVE
		CAHRD	.39.00	186.00	114.55
		MGHED	10.00	159.00	53.45
	· 1	CAMG	0.62	12.90	3.71
		FF	0.01	1.38	0.52
		S04	0.05	1 05.00	24.91
		CL	7.50	25.50	15.09
-		NO3 ·	0.01	2.73	0.49
	20	PH	7.38	8.00	7.69
	20	SPECCOND	155.00	250.00	202.50
±/		DO	18.80	36.60	27.70
	1 2 3 1 10	ALK	190.00	350.00	246.33
		TTLHRD	153.00	350.00	256.00
		CAHPD	100.00	210.00	170.00
		MGHRD	53.00	150.00	86.00
		CAMG	1.33	3.82	2.35
		FE	0.30	2.55	1.11
		S04	37.00	43.00	40.33
		CL	8.00	24.50	17.67
		NO3	1.19	4.80	2.40
	34	PH	7.72	9.04	8.66
1.000	511	SPECCOND	35.00	150.00	78.23
		DO	11.96	57.00	19.68
		ALK	15.00	54.00	43.31
* *		TTLHPD	30.00	129.00	79.31
		CAHED	20.00	98.00	49.00
		MGHRD	10.00	46.00	30.31
		CAMG	1.00	3.16	1.71
		FE	0.01	0.18 .	C.07
	*	SN4	0.00	33.00	9.03
		CL	16.00	45.00	30.65
		N03	0.00	0.60	0.16
	38	FH	7.20	8.52	8.01
	52	SPECCOND	145.00	255.00	196.43
		DO	16.60	22.50	18.25
		ALK	110.00	244.00	139.75
• • • •		TTLHRD	159.00	305.00	211.88
		CAHRD	116.00	552.00	154.88
		MGHRD	40.00	78.00	57.00
		CAMG	1.88	3.68	2.82
		FE	0.09	2.74	1.20
		504	22.30	70.00	38,67
			15.00	26.00	50.00
			0.25	1.20	0.61
	3C	PH	6,94	7.24	7.08
	*****	SPECCUND	125.00	255.00	175.00
			16.10	20.70	18.67
		TTLUDO	110.00	2.37.00	173.50
		CAUDD	135.00	298.00	191.67
		MCHOD	100.00	220.00	144.33
		calic	29.00	78.00	47.33
		EF	2.82	3.90	3.19 .
		504	17.00	7.42	? • 17
		204	11000	39.00	24.93

14

1.4.1

*

162

WELL	PARAM	LC	н	AVE
	CL	7.50	13.00	9.30
	ND3	0.41	6.10	2.31
	PH	6.64	7.61	7.12
4	SPECCOND	325.00	590.00	500.42
	DO	3.22	42.00	17.72
	ALK	123.00	461.00	392.25
	TTLHRD	413.00	660.00	593.08
	CAHRD	249.00	572.00	472.33
	MGHRD	47.00	167.00	121.15
	CAMG	1.52	12.17	4.56
	FE	0.00	0.45 -	0.17
	S04	63.50	140.00	119.46
	CL	41.50	121.00	94.88
	N03	1.30	11.60	5.97
	PH	7.05	8.52	7.84
5	SPECCOND	80.00	310.00	224.85
	00	12.88	43.00	20.60
	ALK	65.00	436.00	230.00
	TTI HRD	80.00	490.00	287.00
	CAHRD	55.00	380.00	217.92
	MGHRD	25.00	110-00	69.08
	CAMG	1.65	11.83	3.57
	EF	0.01	0.78	0.12
	504	11.30	39.50	29.08
	504 Cl	15.00	05.00	27.60
	NOZ	13.00	40.00	0.26
6		7.05	8.70	7.02
0	SPECCOND	87.00	220.00	180.00
	DO	13.80	35.00	19.96
	ALK	82.00	245.00	213.50
	TTLHED	150.00	250.00	215.43
	CAHED	56.00	184.00	154.86
	MGHRD	40.00	90.00	60.57
	CAMG	0.67	4.48	2.79
	FF	0.00	10.57	0.27
	504	3.00	16.00	5.96
	CL	2.00	15.00	5.48
	ND3	0.01	0.66	0.22
	PH	7.08	8.52	7.98
8	SPECCOND	80.00	250.00	195.83
	DO	10.12	20.20	15.22
	ALK	68.00	265.00	206.33
	TTLHRD	147.00	327.00	249.17
	CAHPD	92.00	236.00	184.00
	MGHRD	40.00	91.00	65.17
	CAMG	1.67	5.25	2.93
	FF	0.02	0.98	0.19
	504	10.80	44.00	29.56
	CL	5.00	15.00	10.54
	NO3	0.03	0.27	0.16
0	PH	7.21	8.52	7.88
2	SPECCOND	135.00	265.00	216.43
	DO	9.66	47.00	17.82

a

- E - E

.

6

WELL	PARAM	LO	HI	AVE
	ALK	90.00	. 334.00	275.57
	TTLHRD	120.00	307.00	264.36
	CAHRD	80.00	237.00	181.79
	MGHRD	40.00	138.00	82.57
	CAMG	0.99	3.39	2.28
	FE	0.01	0.65	0.13
	S04	0.00	10.00	7.29
	CL	1.80	9.50	5.40
	NO3	0.12	0.83	0.46
10	PH	7.61	8.39	7.92
	SPECCOND	165.00	240.00	101.67
	DO	15.00	33.40	22.67
	ALK	109.00	298.00	22001
	TTLHRD	168.00	276.00	220.33
	CAHRD	87.00	189.00	126.00
	MGHRD	81.00	87.00	120.00
	CAMG	1.07	01.00	83.67
	FE	0.00	2.17	1.49
	SD4	9.00	10.09	0.39
1 . X	CL	3.50	12.00	10.33
	NOB	0.10	9.50	6.17
	PH	6.00	0.71	0.38
11	SPECCOND	170.00	8.48	7.66
	DD	130.00	380.00	311.46
	ALK	12.07	45.60	16.54
	TTI HOD	85.00	- 545.00	294.08
	CAUDD	125.00	435.00	362.92
	MCHPD	115.00	310.00	259.23
	CAMAG	10.00	140.00	103.69
	CAMO	1.86	11.50	3.22
	FE	0.07	2.50	0.40
	504	19.00	98.00	59.81
		25.00	70.00	33.27
	NUS	1.65	9.80	4.52
12	PH	6.99	8.45	7.64
	SPECCOND	165.00	340.00	227.22
	00	12.70	24.20	16.54
	ALK	60.00	253.00	193.67
	TILHRD	180.00	314.00	244.33
and the second	CAMPD	105.00	230.00	164.67
	MGHPD	40.00	177.00	79.67
	CAMG	0.67	5.75	2.61
	FF	0.00	C . 33	0.08
	SO4	50.00	38.00	27.44
	CL	10.50	22.50	15.50
	NO3	0.04	3.60	. 1.11
13	рн	7.25	8.31	7.69
	SPECCOND	130.00	240.00	191.43
	DO	11.50	. 20.20	16.59
	ALK	211.00	235.00	223.71
	TTLHPD	220.00	268.00	239.71
	CAHRD	144.00	200.00	172.14
	MGHPD	52.00	173.00	67.57
	CAMG	1.60	4.00	2.74

*

÷

WELL	PARAM	LO	HI	AVE
	FE	0.03	0.52	0.18
	504	8.00	21.00	15.67
	CL	5.00	10.00	7.71
	NO3	0.12	6.00	1.10
14	PH	7.30	10.47	8.23
14	SPECCOND	175.00	258.00	230.43
	DO ·	34.60	14.26	20.81
	ALK	260.00	278.00	268.57
	TTLHRD	240.00	315.00	288.86
	CAHRD	175.00	237.00	209.71
	MGHRD	65.00	102.00	79.14
	CAMG	2.09	3.69	2.85
	FE	0.02	0.21	0.06
	SD4	11.50	19.00	1.5.81
	CL	15.00	31.00	19.00
	ND3	0.06	0.39	0.22
15	PH	7.14	8.68	7.93
1.5	SPECCOND	275.00	625.00	425.42
	טס	5.98	19.40	12.51
	ALK	280.00	525.00	394.42
	TTLHRD	75.00	194.00	144.33
	CAHRD	40.00	140.00	84.33
	MGHRD	35.00	82.00	60.00
	CAMG	0.87	2.69	1.43
	FE	0.14	. 2.75	1.26
	S04	18.00	67.00	30.71
	CL	26.50	57.50	39.91
	ND3	0.82	52.00	11.79
16	PH	7.57	8.76	8.11
	SPECCOND	155.00	245.00	216.07
	DO	7.36	20.70	13.99
	ALK	110.00	225.00	180.30
	TTLHRD	250.00	287.00	266.10
	CAHRD	163.00	218.00	187.20
	MGHRD	37.00	118.00	18.90
	CAMG	1.00	5.89	2.02
	FE	0.02	0.00	67 55
	504	44.00	AA 50	19.05
	NOZ	11.00	2.48	0.16
	DH	7.24	8.44	7.90
17	SPECCOND	130.00	255.00	103.43
	DD	6.90	37.40	15.93
	ALK	90.00	217.00	186.21
	TIHRD	140.00	280.00	221.21
	CAHRD	85.00	244.00	162.71
	MGHED	. 30.00	157.00	- 58,50
	CAMG	0.69	7.18	3.33
	FE	0.00	0.58	0.09
	S04	8.00	97.00	36.62
	CL	8.50	15.00	11.21
	NO3	0.02	1.52	0.40
1.9	PH	7.10	8.15	7.60
TO				

*

AN L

34

10.0

1

WELL	PARAM	LO	HI	AVE
	SPECCOND	190.00	205.00	220 44
	DO	6.00	41.00	229.44
	ALK	130.00	213.00	19.34
	TTLHRD	205.00	275.00	276 17
	CAHRD	160.00	218.00	102 25
	MGHRD	17.00	70.00	192020
	CAMG	2.93	12.80	5.21
	FE	0.02	0.75	0.15
	S04	16.00	125.00	65.44
	CL	15.00	25.50	19.11
	NO3	0.11	0.80	0.34
19	PH	7.19	8.74	7.82
	SPECCOND	155.00	315.00	239.62
	00	6.00	31.60	16.64
	ALK	174.00	316.00	266.92
	TTLHRD	20.00	375.00	294.15
	CAHRD	140.00	295.00	219.92
	MGHRD	25.00	161.00	74.23
	CAMG	0.96	10.60	3.89
	FE	0.03	·.23	0.09
	SO4	19.00	150.00	42.44
	CĻ	8.50	19.53	11.54
	N03	0.00	0.68	2.26
21A	PH	•		8.20
	SPECCOND	280.00	220.00	250.00
	DO	11.50	11.04	11.27
	ALK	180.00	216.00	198.00
	TILHRD	240.00	253.00	246.50
	CAHRD	165.00	147.00	156.00
	Cabac	75.00	106.00	90.50
	CAMG	1.39	2.20	1.80
	FF 504	C • 1 1	0.14	0.13
in the second second	5104	50.00	70.00	60.00
	NOT	43.00	52.50	47.75
	DH	0.08	^.20	0.14
21B	SPECCOND	070 00	•	8.00
	DO	2.30.00.	265.00	247.50
	ALK	5.00	14.72	9.89
	TTLHED	200.00	214.00	184.00
	CAHRD	165.20	232.00	216.00
	MGHRD	35.00	175.00	170.00
	CAMG	3.07	07.00	46.00
	FE	0.04	4.71	3.89
	S04	52.00	57.00	0.06
	CL	30.00	37.00	34.57
	NO3	0.24	2.45	33.50
222	PH			7.65
LLA	SPECCOND	295.00	305.00	700.005
	DO	8.28	17.02	12.65
	ALK	216.00	283.00	249.00
	TTLHRD	330.00	380.00	355.00
	CAHRD	250.00	280.00	265.00

14.11

P. 8. 191

1

WELL	PARAM	LO	ні	AVE
	MGHRD	72.00	100.001	86.00
	CAMG	2.80	3.58	3.10
	FF	0.18	0.21	0.20
	SDA	29.00	42.00	75.50
	0	30.00	52.00	33.57
	NOR	0.06	0.08	41.007
	PH	0.00	1.446	8.05
22B	SPECCOND	150.00	210.00	180.00
	DO	6.90	10.58	8.74
	ALK	130-00	270.00	200.00
	TTI HPD	160.00	277.00	218.50
	CAHRD	120.00	275.00	197.50
	MGHRD	2.00	40.00	21.00
	CAMG	3.00	137.50	70.25
	FF	0.15	0.72	0.44
	504	17.00	48.00	32.50
	504 C1	7.50	35.00	21.25
	NOR	0.05	0.11	0.08
	DH	0.05		7.85
23A	SPECCOND	220.00	230.00	225.00
	DO	4.14	8.74	6.44
	ALK	187.00	187.00	187.00
	TTLHRD	244.00	253.00	248.50
	CAHRD	179.00	197.00	188.00
	MGHED	56.00	65.00	62.53
	CAMAG	2.75	3.52	3-14
	EF	0.05	0-11	0.08
	SDA	32.00	53.00	42.50
	CI	22.50	25.00	23.75
	NOR	0.13	C . 20	0.17
	PH			8.16
23B	SPECCOND	190.00	205.00	197.50
	DU	4.60	5.98	5.29
	ALK	186.00	203.00	194.50
	TTLHRD	220.00	226.00	223.00
	CAHRD	157.00	171.00	164.00
	MGHPD	63.00	55.00	59.00
	CAMG	2.49	3.11	2.80
	FE	0.09	. 0.28	0.19
	S04	28.00	30.00	29.00
The Age and Ade	CL	12.50	14.00	13.00
	ND3	0.16	0.24	0.20
STREAM	PH	6.65	8.69	7.88
OINDIN	SPF.CCOND	90.00	270.00	205.00
	סס	6.44	34.40	20.03
	ALK	70.00	233.00	142.83
	TTLHRD	100.00	360.00	215.67
	CAHRD	75.00.	275.00	157.00
	MGHRD	25.00	85.00	58.67
	CAMG .	1.67	4.29	5.83
	FE	0.00	C.09	0.03
	S04	8.00	86.00	36.95
	CL	20.00	39.57	29.38

the second se

24

14

•

WELL	PARAM	LO	HI	AVE
STREAM	NO3	0.15	2 2	
		0.010	8.85	1.86

.



CERRO GORDO COUNTY SITE (E1, Sec. 22, T96N, R21W)

The Cerro Gordo County Landfill near Mason City, Iowa is constructed adjacent to a small upland stream named Crane Creek which runs to the west and north of the site. The surficial material at the site consists of glacial till (silty and sandy clay). This glacial till is normally 20-50 feet thick and contains at least one known lense of sand. The uppermost bedrock unit is the Cerro Gordo Limestone which is rarely used as a source of significant water. The Juniper Hill Shale is approximately 50 feet thick and probably protects the Cedar Valley Limestone, a major regional aquifer (Figure 16).

During the summer of 1976 eight monitor wells were placed on the landfill property. The locations of these wells is shown in Figure 17. Wells cg5a and cg5b serve as background quality

wells and should measure the quality of the ground water before it reaches the landfill. Monitor wells down gradient from the fill include cgla, cglb, cg2, cg3a, cg3b, and cg4. Two stream samples are also collected and analyzed to test the quality of the surface water.

The data collected to date tend to indicate that any leachate plume that may be developing does not extend to the monitor network at this time (Table 19). It could be that the dry climate conditions and the short period of time since operations were begun have not been sufficient for the development of a leachate plume. If the sand lense beneath the site begins to act as a conduit for leachate migration the current monitor network should intercept the plume. During June 1977 a bedrock monitor well was placed into the upper bedrock unit (Cerro Gordo Limestone) to test the quality of water in that horizon. No data is presently available from this new well. A proposed production well for use at the landfill will also serve as a monitor for the Cedar Valley Limestone in the future.







0.001

Well

7.8 7 7 4 1

FIGURE 17.

1.00

TABLE 19

CHEMICAL ANALYSIS OF CERRO GORDO COUNTY SANITARY LANDFILL GROUND WATER MONITORING NETWORK Month December 1976

cg 5b cg 5a cg 3a cg 3b cg 4 cg la cg lb cg 2 PARAMETERS mg/129'1" 26'8" 4'6" 1'9" 15'8" 5'1" 5'11" 6'4" SWL* 9.0 6.0 9.2 8.0 10.2 7.0 8.2 9.5 Co Temp. X 7.6 7.2 7.1. 7.3 7.2 7.3 7.4 pH 220 205 210 205 X 220 220 215 Sp. Cond. .10 .40 .40 1.1 X .60 .42 .25 Fe 15 10 5 10 5 X 5 10 C1 250 190 220 180 X 230 220 220 Ca Hard $(CaCO_3)$ 70 70 80 70 X 60 80 60 Mg Hard $(CaCO_2)$ 320 260 300 250 X 290 300 280 Tot.Hard $(CaCO_2)$ 250 260 280 220 X 310 270 270 Tot.Alk. $(CaCO_2)$ 39 20 29 45 Х 13 11 18 SO4 2.2 .1 .1 .25 X .1 .35 .1 NO3

83

Well Depth 17'9" 25'1" 27'6" 31'7" 18'7" 23'11" 54' 35'

Method of Analysis Hach Method Analysis Performed By Jerry Rick

*SWL= standing water level in feet below surface

X = sample lost or unusable

Conclusions

Although the data generated from the current upland site studies are somewhat limited, several conclusions can be reached:

- 1. The shape and position of pollution enclaves from upland disposal sites are more difficult to predict than flood plain sites due to the natural variability of materials and the large distance to points of ground water discharge. Other activities of man between an upland landfill and the ground water sink may also be responsible for the reduction of water quality.
- 2. The production of leachate in upland positions is dependent on the amount and distribution of precipitation. Enclaves from upland sites therefore form more slowly than those which develop from disposal sites in floodplains where ground water flow is more continuous. Leachate production from upland sites should be periodic, with high concen-

trations associated with flushing following major precipitation events.

3. The theoretical depth of penetration of pollution enclaves from upland sites is greater than those associated with floodplains. The present data, in general, suggests that the concentration of leachate is greatest in the refuse and in nearby shallow materials. Water samples from deeper wells show that leachate concentrations decrease with distance away from the site and with increasing depth. This could be due to either a lack of production of significant amounts of leachate, or the attenuation of leachate as it moves through the ground.

4. Limited initial data from the three upland sites indicate that leachate concentrations are lower than those found in previous investigations on floodplain sites (Peckenpaugh, 1973; Stevens, 1974). This may be a direct result of the topographic position with its associated ground water regimen. The relative ages of the sites tested may also be important. All of the upland landfills which were studied are fairly new and have been operated as sanitary landfills since the deposition of wastes was begun on the sites. The floodplain sites which were tested were much older and had a complex history of open dumping.

The present study of upland landfills should be continued. An effort to determine the long term effects of these sites on the quality of ground and surface water in the surrounding regions should be made. It should be recognized that leachate production in upland sites is dependent on precipitation and therefore may be periodic. Longer periods of study may be necessary to generate the data needed for a proper evaluation of the use of upland sites for waste disposal.

REFERENCES CITED

- Clark, T. P. 1975. Survey of ground-water protection methods for Illinois landfills. Ground Water. V. 13, No. 4, pp. 321-331.
- Drake, L. Iowa State Water Resources Research Institute Annual Report, 1972.
- Fenton, T. Iowa State Water Resources Research Institute Annual Report, 1973.
- Iowa Highway Research Board. Drainage Area of Iowa Streams. Bull. No. 7.
- Kaufman, R. F. 1970a. Hydrogeology of solid waste disposal sites in Madison, Wisconsin. Ph.D. thesis, University of Wisconsin.
- Kaufman, R. F. 1970b. The role of hydrogeology in the regulation of solid waste disposal and site selection. Geol. Soc. of America. Abstracts with Programs. V. 2, No. 7, pp. 591-592.
- Klefstad, G. E. 1973. Limitations of the electrical resistivity method for detecting landfill leachate in alluvial deposits. Unpub. M.S. thesis Iowa State University, Ames, Iowa.
- Klefstad, G. E., L. V. A. Sendlein, and R. C. Palmquist. 1975. Limitations of the electrical resistivity method in landfill investigations. Ground Water. V. 13, No. 5, pp. 418-427.
- Kunkle, G. R. 1976. Monitoring ground water quality near a sanitary landfill. Ground Water. V. 14. No. 1, pp.11-20.

LeGrand, H. E. 1965. Patterns of contaminated zones in the ground. Water Resources Research. V. 1., No. 1, pp. 83-95.

Palmquist, R. C. and L. V. A. Sendlein. 1975. The configuration of contamination enclaves resulting from refuse disposal sites on floodplains. Ground Water. V. 13, No. 2, pp. 167-181.

Peckenpaugh, J. M. 1973. Aluvial ground-water quality alteration as related to solid waste disposal sites in Iowa. Unpub. M.S. thesis Iowa State University, Ames, Iowa.

Sendlein, L. V. A. and R. C. Palmquist. 1975. A topographic-hydrogeologic model for solid waste landfill siting. Ground Water, V. 13, No. 3, pp. 260-268.

- Sendlein, L. V. A. and R. C. Palmquist. 1973. A geohydrologic model for disposal site evaluation. Geol. Soc. America. Abstracts with Programs. V. 5, No. 4., pp. 350.
- Stephens, M. R. 1974. The shape of ground water contamination zones induced by solid waste disposal sites located on floodplains. Unpub. M. S. thesis, Iowa State University, Ames, Iowa.
- Toth, J. 1963. A theoretical analysis of ground water flow in small drainage basins. Jour. Geophys. Research. V. 68, pp. 4795-4811.
- Tuthill, S. J., D. L. Gordon and F. H. Dorheim. 1972. Hydrogeologic considerations in solid waste storage in Iowa. Iowa Geological Survey Public Information Circular No. 4, pp. 59.

OTHER SELECTED REFERENCES

- Sendlein, L. V. A. and R. C. Palmquist. 1974. The configuration of contaminated ground water zones resulting from solid waste disposal sites. Technical Education Sessions, Annual Meeting, National Water Well Association. Denver, Colorado (Abstract).
- Sendlein, L. V. A. and R. C. Palmquist. 1974. The shape of malenclaves resulting from disposal sites in alluvium. Annual Meeting Geol. Soc. America. Miami Beach, Florida (Abstract).





