

# A d d e n d u m

Iowa Department of Transportation  
Office of Contracts

Date of Letting: May 21, 2013  
Date of Addendum: May 1, 2013

<b>B.O.</b>	<b>Proposal ID</b>	<b>Proposal Work Type</b>	<b>County</b>	<b>Project Number</b>	<b>Addendum</b>
113	78-0801-367	PCC PAVEMENT - GRADE & REPLACE	POTTAWATTAMIE	IM-NHS-080-1(309)2--03-78 IM-NHS-080-1(367)2--03-78 IM-NHS-080-1(368)2--03-78 IM-NHS-080-1(369)2--03-78	21MAY113.A02

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Notice: Only the bid proposal holders receive this addendum and responsibility for notifying any potential subcontractors or suppliers remains with the proposal holder.

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Make the following changes to the PROPOSAL SCHEDULE OF PRICES:

Change Proposal Line No. 0700 2416-1262024 CULVERT, CONCRETE PIPE, 2000D,  
TRENCHLESS, 24 IN. DIA.:

From: 618.000 LF

To: 690.000 LF

Change Proposal Line No. 0860 2503-0114224 STORM SEWER GRAVITY MAIN,  
TRENCHED, REINFORCED CONCRETE PIPE (RCP), 2000D (CLASS III), 24 IN.:

From: 3,149.000 LF

To: 3,313.000 LF

Delete Proposal Line No. 0920 2503-0200036 REMOVE STORM SEWER PIPE LESS THAN  
OR EQUAL TO 36 IN.; 5,400.000 LF

Add Proposal Line No. 1401 2107-0875000 COMPACTION WITH MOISTURE AND  
DENSITY CONTROL; 794.000 CY

If the above changes are not made, they will be made as shown here.

Make the following changes to the PROPOSAL:

Add:

DS-12022      October 16, 2012  
MOISTURE AND DENSITY TESTING FOR SPECIAL COMPACTION OF  
SUBGRADE

SP-120062      May 21, 2013  
TEMPORARY LEVEE CONSTRUCTION  
Note: SP-120062 is attached.

Delete:

DS-12024      October 16, 2012  
COMPACTION WITH MOISTURE AND DENSITY CONTROL

Make the following changes to plan set IM-NHS-080-1(367)2--03-78:

Plan Sheet D.6:

Delete the following pipe note:

Sta. 7456+49.00  
Jack 24" x 206' RCP to existing intake  
FL= Lt. 973.02  
Rt. 967.00

Plan Sheet J.1:

For Stage 2A "Grade and Pave detour 251" shall be changed to "Grade and Pave Detour 254"

For Stage 3, replace "Detour 251" with "Detour 254" and delete line "Grade and Pave Detour 254" – Note this was already built in stage 2A

For Stage 5, delete reference to "Detour 251"

Sheet J.24:

Change Detour 251 to Detour 254

Plan Sheet M.1:

For Line Number P L-35, the Pipe Diameter shall be 24"



**SPECIAL PROVISION  
FOR  
TEMPORARY LEVEE CONSTRUCTION**

**Pottawattamie County  
IM-NHS-80-1(367)2--03-78**

**Effective Date  
May 21, 2013**

**THE STANDARD SPECIFICATIONS, SERIES 2012, ARE AMENDED BY THE FOLLOWING MODIFICATIONS AND ADDITIONS. THESE ARE SPECIAL PROVISIONS AND THEY SHALL PREVAIL OVER THOSE PUBLISHED IN THE STANDARD SPECIFICATIONS.**

**120062.01 DESCRIPTION.**

- A. The work covered by this Special Provision consists of furnishing all labor and materials, and performing all operations in connection with the construction of the temporary impervious levee adjacent to the east abutment, as shown in the Contract Documents.
- B. The geotechnical information for the levee construction is provided in the document entitled "Modifications to Council Bluffs Levee Unit II – Sections 2 and 3, Proposed I-29/I-80 Bridges over Indian Creek, Council bluffs Interstate System – Segments 2 and 3, Project No. IM-029-2(55)49—13-78, Pottawattamie County, Iowa," prepared by HDR, Inc., dated February 22, 2013. This document is available from the Office of Contracts.

**120062.02 MATERIALS.**

- A. **General.**  
Construct the temporary levee entirely of impervious fill obtained from approved off site borrow areas. Fill used in construction of embankments shall be free of unsuitable materials.
- B. **Impervious Fill.**  
Impervious fill shall consist of cohesive materials having at least 50% passing the U.S. Standard 200 mesh sieve size. Cohesive materials include materials classifying as lean (CL), having a Plasticity Index of 10 or greater, and falling between the "U" line and the "A" line on Figure 3 in ASTM D 2487 – Standard Tests for Classifications of Soils for Engineering Purposes.
- C. **Unsuitable Materials.**  
Unsuitable materials are materials containing debris, brush, roots, sod, organic matter or stones with dimensions greater than one-half the loose layer thickness and shall not be used in the levees. Frozen earth, snow, or ice shall not be used in the levees. In addition, silts and sands are considered unsuitable.

**D. Pre-Construction Testing of Proposed Borrow Materials**

Submit to the Engineer for approval the results of grain size tests (ASTM D 422) and plasticity tests (ASTM D 423 and D 424) for the impervious fill. The estimated quantity and location of materials proposed for use in the levee embankment shall also be submitted. These submittals must be approved by the Engineer prior to the placement of materials within the levee section.

**120062.03 CONSTRUCTION.**

**A. Notifications.**

Notify the City of Council Bluffs and the Engineer 1 week prior to construction of the temporary impervious levee and at the completion of the temporary impervious levee construction, at least 1 week prior to beginning any excavations within the existing levee section.

**B. Subgrade Preparation.**

**1. General.**

After stripping and removal of unsuitable foundation material, cavities or other depressions shall be broken down to flatten out the slopes. Immediately prior to the placement of fill material for the temporary impervious levee, the entire earth surface on or against which fill is to be placed, shall be thoroughly broken to a depth of 6 inches and the area to be occupied by the embankments shall be compacted in accordance with the provisions specified in Article 120062.03, C. If for any cause, this foundation surface or other fill surface that is to receive fill becomes compacted in such a manner that, in the opinion of the Engineer, a plane of seepage or weakness might be induced, it shall be scarified to a depth of 6 inches before the depositing of material thereon.

**2. Requirements.**

The depth of stripping on the existing slopes of levees and roadway embankments shall not exceed 6 inches. All slopes upon which fill is to be placed shall be notched and broken up so that the fill material will bond with the existing material. Water shall be added as needed to achieve compaction requirements. All notching, scarifying, and breaking of ground surface shall be done parallel to the centerline of the embankment being constructed. All foundation preparation shall be approved by the Engineer prior to placement of embankment. No fill shall be placed upon frozen ground.

**3. Existing Embankments.**

At locations where the levee or other embankments tie to existing earth embankments to complete the temporary impervious levee, such embankment surface slopes shall be notched, scarified and compacted as specified herein. When directed by the Engineer, such operations shall be accomplished parallel to the axis of the existing embankment.

**C. Embankment Construction and Testing.**

**1. General.**

Apply Section 2107 of the Standard Specifications, except when amended by requirements of this specification. Verify embankment placed with moisture and density control meets the requirements of Article 2107.03, I of the Standard Specifications.

Fill material for the temporary impervious levee, as specified in Article 120062.02, shall be placed in successive horizontal layers not more than 8 inches in depth prior to compaction. Each layer shall be spread uniformly on the previously compacted surface; plowed, disked, or otherwise broke up; moistened or aerated as necessary; thoroughly mixed and compacted to produce embankments having the following moisture and density requirements. If in the opinion of the Engineer, the surface of any compacted layer is too smooth to bond properly with the succeeding layer, it shall be scarified to the satisfaction of the Engineer before the

succeeding layer is placed thereon. The Contractor must use equipment which achieves the compaction specified below and which will not create seams between embankment lifts.

**2. Quality Control Program (Embankment Construction).**

Provide and maintain a Quality Control Program (Embankment Construction), defined as all activities of sampling, testing, process control inspection, and necessary adjustments for construction of embankments to meet the requirements of this Special Provision.

As part of the Quality Control Program (Embankment Construction), provide a Quality Control Technician to perform testing on all embankment soils placed with Compaction with Moisture and Density Control. As a minimum, the Quality Control Technician shall have a high school education. The Technician shall obtain 'Soils Technician Lab Certification' through the Iowa DOT's Technical Training and Certification Program (TTCP). TTCP Requirements of Materials I.M. 213 apply.

Ensure the Quality Control Technician is present on the project when embankment is being placed with 'Compaction with Moisture and Density Control.'

Provide a laboratory facility and all the necessary calibrated equipment to perform the required tests.

**3. Test Procedures.**

Use test procedures and equipment complying with applicable Materials I.M.'s, Iowa DOT Materials Laboratory Test Methods, or equivalent standards of AASHTO or ASTM.

Allow the Engineer to review equivalent standards. Use equivalent standards only if approved by the Engineer.

Acceptable test methods for determining moisture content and density are:

- Oven drying - AASHTO T 265
- Pan drying - ASHTO T 265 (modified to use an open burner)
- Microwave - ASTM D 4643
- Nuclear gauge - Materials I.M. 334
- Density of soil cores - Materials I.M. 326
- Sand Cone Test - ASTM D 1556

Use AASHTO T 265 oven drying method for the reference method for calibration.

Atterberg Limits (ASTM D 4318) and grain size analysis (ASTM D 422) shall also be determined for each of the representative materials. Copies of all test results made for and used as a basis for moisture and density control shall be furnished to the Engineer in advance of the time that materials are to be placed in the embankments.

**4. Moisture and Density Control.**

Moisture and density control of the embankments shall be based on the standard Proctor compaction test (Materials I.M. 309). Cohesive materials placed in embankments shall be compacted to a density of at least 95% of the maximum dry density and be within -1% to +4% of the optimum moisture content at the time the compactive effort is applied which may require the addition of water or aeration of materials. Non-cohesive materials used in the embankments will be placed in a moist condition and compacted with approved equipment to a density of at least 95% of the maximum dry density.

Determine the optimum moisture content and maximum density by Proctor testing of soil being placed. Determine optimum moisture and maximum density for each type of excavated or mixed soil which varies as to change the expected AASHTO classification, or if directed by the Engineer.

Test and verify that moisture content and density of material placed under the item 'Compaction with Moisture and Density Control' is within the optimum moisture content range for the soil being placed and greater than or equal to the required density.

Disk to reduce moisture if, after initial disking to break down lumps greater than 12 inches as required by Article 2107.03, D, 2, d, of the Standard Specifications, the deposited soil material contains moisture in excess of the specified moisture limits.

If, after initial disking as required by Article 2107.03, D, 2, d, of the Standard Specifications, the material is dry to the extent that it is not within the range of the optimum moisture of the soil to allow satisfactory compaction by rolling, uniformly moisten the material to the required limits before it is compacted.

Proceed with aeration, watering, and compaction operations in an orderly fashion without unreasonable and unnecessary delay. Compensation will not be allowed for delays resulting from the ordering of moistening or disking.

Verify all soil accepted for final placement is within the specified moisture control limits and meets the in-place density requirements.

**5. Compaction.**

Apply Article 2107.03, E, of the Standard Specifications.

**6. Equipment.**

Apply Article 2107.03, B of the Standard Specifications, except that for compaction of granular sand soils classified as AASHTO A-1, A-2, or A-3 and having 15% or less combined silt/clay content (percent passing the No. 200 (75 µm sieve) use:

- Pneumatic tired rollers as described in Articles 2001.05, C and 2001.05, D of the Standard Specifications, or
- Self-propelled vibratory rollers as described in Article 2001.05, F of the Standard Specifications.

**7. Test Frequency during Embankment Construction.**

Test for proctor optimum moisture content and maximum density and embankment moisture content and density at the minimum frequencies in Materials I.M. 204. Samples will be randomly selected.

**8. Field Records.**

Document all observations, records and inspection, changes in soil type, soil moisture, fill placement procedures, and test results on a weekly basis. Note the results of the observations and records of inspection in a permanent field record as they occur. Submit copies of field moisture and density tests to the Engineer on a weekly basis. Submit the original testing records (raw field and lab data sheets) and control charts to the Engineer in a neat and orderly manner within five calendar days after completion of the project.

**9. Corrective Action.**

Notify the Engineer when a moisture content falls outside the specified control limits or density falls below the required minimum. If a single moisture content falls outside of the control limits, the fill material in this area will be considered unacceptable for compaction. Perform corrective action(s) to bring the uncompacted fill material, after a retest, within the specified moisture control limits. If material has been compacted, disk it, bring it to within moisture control limits, and recompact it. Also, if a single density does not meet the requirements, the compacted fill material in this area will be considered unacceptable. Perform corrective action(s) to the material to meet density requirements.

**10. Quality Assurance.****a. Required Testing.**

The Contractor's Quality Control Technician shall perform all field testing and data analysis. The Quality Control Technician shall retain split samples of Materials I.M. 309 testing when requested by the Engineer. The Engineer may select any or all of the Contractor-retained split samples for independent assurance and verification testing.

The Engineer will determine the random location of verification tests and will test at the minimum frequencies in Materials I.M. 204. The Contractor Quality Control Technician shall obtain a sample at the same location as directed by the Engineer and provide the results to the Engineer. Verification test results will be provided to the Contractor within one working day after the Contractor's quality control test results have been reported.

The Engineer will periodically witness field testing being performed by the Contractor. If the Engineer observes the quality control field tests are not being performed according to the applicable test procedures, the Engineer may stop production until corrective action is taken. The Engineer will notify the Contractor of observed deficiencies, promptly, both verbally and in writing. The Engineer will document all witnessed testing.

All quality control test-results become part of the project files.

**b. Verification and Independent Assurance Testing.**

The Contractor's quality control test results will be validated by the Engineer's verification test results using the criteria in Materials I.M. 216. If the Engineer's verification test results validate the Contractor's test results, the Contractor's results will be used for material acceptance.

In the event that the Contractor's results can't be validated, the Engineer will investigate the reason immediately. The Engineer's investigation may include:

- Testing of other locations,
- Observations of the Contractor's testing procedures and equipment, and
- Comparison of test results of the Contractor with those of the Engineer.

Personnel and laboratories performing tests used in the acceptance of material shall participate in the independent assurance program covered in Materials I.M. 205.

**c. Referee Testing.**

If a difference in procedures for sampling and testing and/or test results exists between the Contractor and the Engineer which they cannot resolve, the Iowa DOT's Central Materials Laboratory will provide referee testing. The Engineer and the Contractor will abide by the results of the referee testing.

**11. Acceptance.**

The Engineer will base final acceptance of tests and materials on the results of the Contractor's quality control testing as verified by the Engineer's quality assurance.

**120062.04 METHOD OF MEASUREMENT.**

- A.** The quantity of Contractor furnished impervious fill will be measured in cubic yards placed and paid for as Embankment-In-Place.
- B.** The quantity of embankment requiring Compaction with Moisture and Density Control, in cubic yards, will be the quantity shown in the contract documents as determined by the template fill volume. Shrinkage will not be included in moisture and density control quantity.

- C. All excavation in preparation for and construction of embankment with moisture and density control will be included in Class 20 Excavation according to Article 2402.04 of the Standard Specifications.

**120062.05 BASIS OF PAYMENT.**

- A. The quantity of Embankment-In-Place accepted for payment will be paid for at the contract unit price. The contract unit price will be full and complete payment for providing all design, materials, labor, equipment, and incidentals to complete the work.
- B. Payment for Compaction with Moisture and Density Control will be the contract unit price in cubic yards for the quantity of embankment placed with moisture and density control.

Payment is full compensation for furnishing a Quality Control Technician, sampling and testing, process control inspection, working of drying material, furnishing and applying water, controlling moisture content of the materials, and compacting the materials to the required density, as specified.