K. Jeyapalan, W. Yao, S. Savathchandra, N. Narayan, S. McMahon, J. Kang

Airborne GPS

Final Report

April 1995

Sponsored by the Iowa Department of Transportation and the Iowa Highway Research Board

> Iowa DOT Project HR-359 ISU-ERI-Ames-95162



The opinions, findings, and conclusions expressed in this publication are those of the authors and not necessarily those of the Iowa Department of Transportation.

K. Jeyapalan, W. Yao, S. Savathchandra, N. Narayan, S. McMahon, J. Kang

Airborne GPS

Final Report

April 1995

Sponsored by the Iowa Department of Transportation and the Iowa Highway Research Board

> Iowa DOT Project HR-359 ISU-ERI-Ames-95162



Executive Summary
1. Introduction
2. Photogrammetry and Kinematic GPS
2.1. Photogrammetry
2.2. Kinematic GPS
2.3. Application of Kinematic GPS in Photogrammetry
3. Analysis of First Test
4. Analysis of Second Test
5. Analysis of Third Test
6. Analysis of Final Test
6.1. ISU and Highway 30 Project
6.2. Ground and Flight Control
6.3. Flight Mission
6.4. Processing of GPS Data
6.5. Photo Coordinate Observation
6.6. Analysis of the Flight Data
6.7. Analysis of Refined Test Flight Data
7. Applications of Airborne GPS
7.1. Rectifying Aerial Photos
7.2. Producing Orthophotos
7.3. Stereo Plotting
7.4. Base Components
7.5. ϕ and ω Angles for Leveling
7.6. Orientation Angle and Translation
8. Conclusion and Recommendation
9. Acknowledgments
10. Bibliography
Appendix. Processing of GPS Data for 1994 Project

CONTENTS

GRAPHS

Graph 3.1.	Camera-wing Z difference vs. time
Graph 3.2.	St. Louis test 1 (photogrammetry) vs. GPS
Graph 3.3.	Original and filtered height
Graph 5.1.	Flight path of Surdex aircraft
Graph 5.2.	Position solution for tail antenna for flight #1
Graph 5.3.	Comparison of ω values-high flight
Graph 5.4.	Comparison of ω values–low flight
Graph 5.5.	Original and filtered C-R
Graph 5.6.	Change in length of wing with time
Graph 5.7.	Spec for high flight-high frequency
Graph 5.8.	Spec for high flight-low frequency

FIGURES

Figure 2.1.	Coordinate system
Figure 2.2.	Stereo plotter
Figure 2.3.	Global positioning system
Figure 2.4.	Interferometric method
Figure 2.5.	Z-12 technical specifications
Figure 2.6.	Ashtech Z-12 tm GPSreceiver
Figure 2.7.	Multiantenna locations
Figure 2.8.	Triplet
Figure 3.1.	Aircraft used in tests
Figure 3.2.	Aircraft dimension
Figure 3.3.	Camera and receiver inside aircraft
Figure 3.4.	St. Louis GCP flight plan
Figure 4.1.	Campus flight plan diagram
Figure 4.2.	Low-flight campus flight plan
Figure 5.1.	Aircraft with four antennas
Figure 5.2.	Change in scale of wings
Figure 6.1.	ISU campus control
Figure 6.2.	Highway 30 flight, 1994
-	Highway 30 control
Figure 6.4.	Airport control
Figure 6.5.	GPS receiver arrangement in the aircraft
Figure 6.6.	GPS data collection on photo mission
Figure 6.7.	Project 1994
Figure 6.8.	Campus flight 1994
Figure 6.9.	Base station GPS data collection
Figure 6.10	Antenna locations at taxi point
Figure 6.11	. Computation of ϕ_G,ω_G,k_G
Figure 6.12	2. Sources of error in ω_P
Figure 7.1.	Rectification
Figure 7.2.	Orthophoto production

TABLES

Table 3.1.	Camera antenna position
Table 3.2.	Wing antenna position
Table 3.3.	Comparison of ω by GPS and photogrammetry
Table 4.1.	Camera location from navigation software
Table 4.2.	Comparison of GPS navigation and photogrammetry
Table 5.1.	Camera antenna position
Table 5.2.	Right wing antenna position
Table 5.3.	Tail antenna position.
Table 5.4.	Omega, phi, kappa, and scale by GPS
Table 5.5.	Comparison of ω values
Table 5.6.	Comparison of exterior orientation elements
Table 6.1.	Results of flight 1 for entire time
Table 6.2.	Data of flight 1 for exposure time
Table 6.3.	Photo-GPS location
Table 6.4.	GPS-Photo orientation
Table 6.5.	Results of combination of high and low flights
Table 6.6.	ω_G to w_p using different weights
Table 6.7.	GPS-Photo locations using refined data
Table 6.8.	ω_G to ω_p using refined data
Table 6.9.	First and second difference in $\omega_p - \omega_G$
Table A-1	. Data of flight 1 for entire time
Table A-2	. Results of flight 1 for entire time 103

EXECUTIVE SUMMARY

The airborne Global Positioning System (GPS) research project began in April 1993; a series of four tests were carried out in St. Louis, Missouri and in Ames, Iowa. All of the tests, except one, were performed in cooperation with Ashtech, a GPS firm located in Sunnyvale, California and Surdex Inc., a photogrammetric firm located in St. Louis, using Cessna aircraft, LMK 2000 cameras, and Ashtech receivers. The photo coordinates were observed using a Wild STK1 stereo comparator and were processed using Sat9, RO, Albany, and Calib softwares. The GPS data were processed using GPPS and PNAV software. The objective of this research project was to use a GPS to determine the best aerial camera location and orientation for mapping.

In the first test, the camera antenna and left wing antenna were mounted on the aircraft, which was flown over the St. Louis site. The test proved that observations can be taken using wing and camera antennas and that wing motion can be modeled to get the ω rotation.

In the second test, the navigation antenna was mounted on the aircraft's fuselage. A trimble C/A code receiver was used with real-time photo mission navigation software. In a flight over the Iowa State University (ISU) campus test site, Aerial Services Inc. took photographs. This test proved that pinpoint navigation is feasible in the x-y direction and has an accuracy of ± 25 meters. Because the C/A code was used in real time, the accuracy may be about ± 50 meters in the z direction, which can be avoided by using either a P code GPS receiver or the usual on-board aneroid barometer.

In the third test, four antennas were used: camera, left wing, right wing, and tail. In this test flight over the St. Louis site, two GPS L_1/L_2 P12 receivers and one 3DF GPS receiver were used. The test proved that the tail antenna is not suitable due to multipath, that the 3DF GPS receiver is not suitable for airborne GPS applications because it is an L_1 GPS receiver, and that at least seven satellites are needed for reliable PNAV solutions.

In the final test, one navigation antenna; four airborne GPS antennas: camera, left wing, right wing, and forward; four Z12 receivers on-board; and two Z12 receivers on the reference stations were used. This test confirmed that (1) photo coordinates have to be observed two or more times to eliminate small errors, (2) ground elevations established by GPS may have ± 10 centimeters errors because of local geoid undulation, (3) the photographic site has to be within 10 kilometers of the reference base station, (4) the camera antenna coordinates have to be corrected for geoid undulation, and (5) the accuracy of the Z12 is 0.2 millimeters, which neglects the multipath, resulting in the accuracy of ± 0.0001 radians or better in the ω angle.

In summary, the project showed that airborne GPS is feasible for aerial camera location and orientation. In block triangulation, no ground control is required if the site is within 10 kilometers of the reference base station. In a strip, a self calibration is required to transform ω_{G} to ω_{p} and the

calibration site is within 10 kilometers of the photographic site or the height differences between two or more ground control points in the direction perpendicular to the flight are known.

The project was conducted by the Engineering Research Institute of ISU with funding provided by a grant from the Iowa Highway Research Board.

1. INTRODUCTION

A Global Positioning System (GPS) can be used in different applications. The objective of this project, Airborne GPS, was to use a GPS to determine the aerial camera location and orientation that best facilitated mapping done from aerial photographs without any ground control.

In the period April 1993 to April 1995, K. Jeyapalan, Wu Yao, S. D. Savathchandra, Nadella V. Narayan, Scott M. McMahon, and Jingfeng Kang organized this research, conducted four test flights, and analyzed the data. The first test flight was performed in June 1993 at St. Louis, with the objective of testing the multiantenna concept using two antenna on the aircraft. The second test in August 1993 was conducted over the Iowa State University (ISU) campus at Ames. This flight evaluated the use of GPS for pinpoint navigation. The third test flight over St. Louis was flown in October 1993, with four antenna on aircraft; its objective was to evaluate the 3DF GPS receiver and the antenna locations.

On the basis of these three test results, a final test flight over the Mustang Project area in Ames and the ISU campus was conducted in June 1994. Analysis of these data showed that airborne GPS can be used (1) in pinpoint navigation with an accuracy of 25 meters or better, (2) to determine the location of the camera nodal point with an accuracy of 10 centimeters or better, and (3) to determine the orientation angles of the camera with an accuracy of 0.0001 radians or better.

In addition, the exterior orientation elements determined by airborne GPS can be used to rectify aerial photos, to produce orthophotos, and in direct stereo plotting. Further research is recommended in these areas to maximize the use of airborne GPS.

Previous reports [34, 35] have discussed in detail the theory of GPS and photogrammetry. Also the previous reports gave details of the software—Sat9, RO, Albany, Geolab, GPPS, and Calib—used in this project except for PNAV. Ashtech Inc. developed the PNAV software, which is briefly described in the appendix.

The work performed for this research project and its conclusions and recommendations are in the following chapters:

2. Photogrammetry and kinematic GPS

3. Analysis of first test

4. Analysis of second test

5. Analysis of third test

6. Analysis of final test

7. Applications of airborne GPS

8. Conclusions and recommendations

2. PHOTOGRAMMETRY AND KINEMATIC GPS

References [34,35] give detailed information about photogrammetry and GPS. The objective of this chapter is to describe briefly photogrammetry and kinematic GPS as they relate to airborne GPS.

2.1. Photogrammetry

In photogrammetry the photo coordinates (x, y) are related to the ground coordinates (X_G, Y_G, Z_G) (see Fig. 2.1) by the following equation:

$$x - x_o = f(a11(X_G - X_o) + a12(Y_G - Y_o) + a13(Z_G - Z_o)) / (a31(X_G - X_o) + a32(Y_G - Y_o) + a33(Z_G - Z_o)) + radial distortion + decentering distortion + refraction$$

$$y - y_o = f\left(a21(X_G - X_o) + a22(Y_G - Y_o) + a23(Z_G - Z_o)\right) / (a31(X_G - X_o) + a32(Y_G - Y_o) + a33(Z_G - Z_o)) + \text{ radial distortion } + \text{ decentering distortion } + \text{ refraction}$$

where x_0 , y_0 , f are interior orientation elements, (X_0 , Y_0 , Z_0) are the nodal point coordinates in the ground coordinates system (see Fig. 2.1). and

$$A = R_k R_{\phi} R_{\omega} = \begin{pmatrix} a11 & a12 & a13 \\ a21 & a22 & a23 \\ a31 & a32 & a33 \end{pmatrix}$$

Where R_K , R_{ϕ} , R_{ω} are the rotation matrix required to make the photo coordinates axes (x,y,z) parallel to the ground coordinate axes by rotating first about x axis by ω , then about y axis by ϕ , and finally about z axis by K. The K, ϕ , and ω are known as the orientation angles. The X_o, Y_o, Z_o, K, ϕ , and ω are known as the exterior orientation elements.

The objective of photogrammetry is to determine (X_G, Y_G, Z_G) of a point from the photo coordinates of two or more photographs. This is done by three methods: analog, analytical, and self calibration.

In the analog method, the interior orientation and radial and decentering distortions are assumed to be small. The projectors are used to project the images and produce the stereo models (see Fig. 2.2). When producing the stereo model, five of the twelve exterior orientation elements

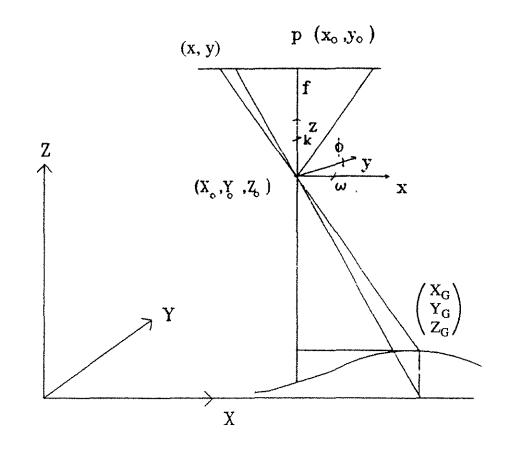


Figure 2.1. Coordinate system.

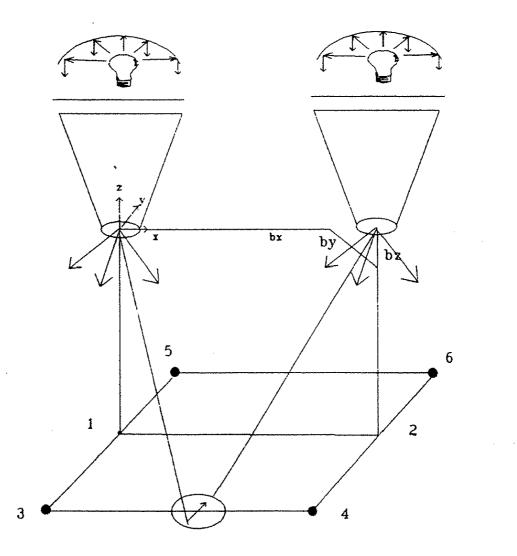


Figure 2.2. Stereo plotter.

,

are determined by relative orientation. The stereo model is scaled and leveled using external ground control points, determining the other seven exterior orientation elements. Special instruments such as a Zeiss Z8 are designed to produce the stereo model and then plot the map.

In the analytical method, the photo coordinates are corrected for interior orientation and for radial and decentering distortions given by the calibration of the camera. The photo coordinates of two or more photos, together with three or more known ground controls, are simultaneously adjusted to give the ground coordinates. Software such as Albany is capable of such adjustment. Some stereo plotters, which are connected to computers for doing these computations in real time and which assist in driving the plotters, are known as analytical plotters.

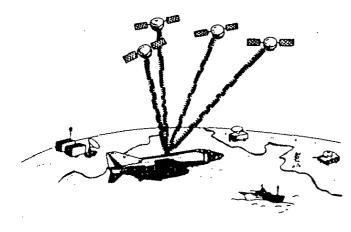
In self calibration, the interior orientation elements, the radial and decentering lens distortion elements, and the exterior orientation elements are simultaneously determined with unknown ground control points using the photo coordinates of two or more photos and a number of ground control points. The method used is normally the least-squares constraint method in which any of the parameters are constrained to its known accuracy. The program such as Calib is capable of this adjustment.

2.2. Kinematic GPS

The GPS consists of 24 satellites orbiting about 20,000 kilometers above the earth (see Fig. 2.3). The satellites transmit information in two carrier frequencies L_1 and L_2 modulated by two codes P and C/A code.

Differential GPS tracks the same satellites from two stations. Using the carrier phase frequency, the base line vector can be computed accurately (see Fig. 2.4). The accuracy depends on the accuracy of the phase measurement, error due to the multipath, and the ionospheric error depending on the distance between the two stations. The use of P and C/A code may eliminate the multipath, and use of L_1 and L_2 may eliminate the ionospheric error. The Z12 Ashtech receiver measures the phase to an accuracy of 0.2 millimeters and has the capability of tracking L_1 and L_2 frequencies (see Fig. 2.5).

In kinematic GPS one of the receivers is fixed at the base station and the other is free to move. The phase angle from each satellite is measured continuously. However, only portions of the phase angle less than 2π are measured at one time; hence the receiver has to keep track of the total phase angle and the integer number of 2π . When a receiver moves, it may lose track of a satellite and lose the integer number of 2π . Knowing the position of the base receiver and the position of the rover, using the other satellites, the lost integer count can be calculated. The PNAV software is capable of resolving the integer ambiguity on the fly, provided there are more than seven satellites at a time (see Fig. 2.6)



CHARACTERISTICS

- SIGNALS: L1 1575 MHz (P & C/A CODE)
 - L₂ 1227 MHz (P or C/A CODE)
- EXPECTED ACCURACY (90%)

	P	SIGNAL	C/A SIGNAL
POSITION	0.1	FT	< 300 FT
VELOCITY		FT/SEC	< 1 FT/SEC
TIME		NS	< 100 NS

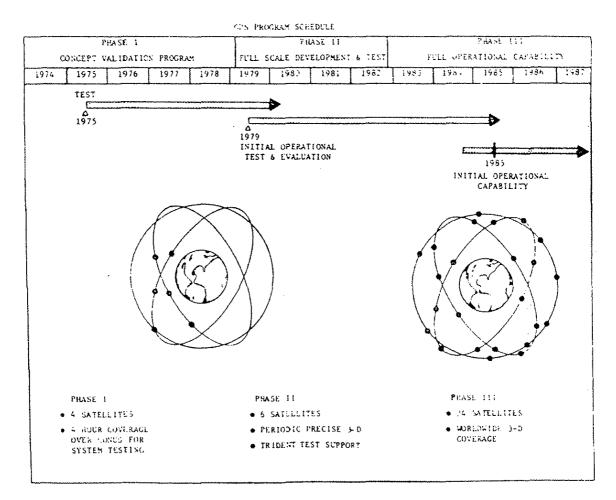


Figure 2.3. Global positioning system.

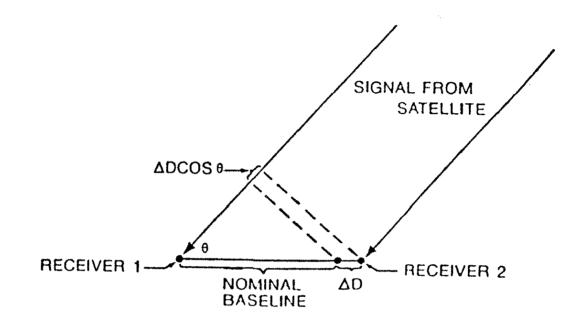


Figure 2.4. Interferometric method.

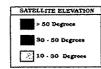
(

Z-12 Technical Specifications

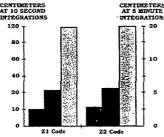
Measurement Precision

C/A (>25°)						
Carrier phase	(25 Hz)	0.15 cm				
	(1 sec)	0.02 cm				
Pseudo-range	(10 sec) ·	20.00 cm				
	(5 sec)	3.60 cm				
P-Code A/S Off (:	>25°)					
L1 Carrier phase	(10 sec)	0.10 cm				
	(5 min)	0.02 cm				
L1 Pseudo-range	(10 sec)	5.00 cm				
	(5 min)	0.90 cm				
L2 Carrier phase	(10 sec)	0.10 cm				
	(5 min)	0.02 cm				
L2 Pseudo-range	(10 sec)	7.00 cm				
	(5 min)	1.30 cm				
Real-Time Differe	ntial Positi	on <im< td=""></im<>				
	(PDOP </td <td>\$)</td>	\$)				
Statis, Rapid Static or 5 mm + 1 ppm						
Pseudo-Kinematic Survey						

P-Code A/S On (Z-Tracking)



OBSERVABLE RMS IN CENTIMETERS FOR THE Z-12 CENTIMETERS AT 10 SECOND



Systematic Errors (Between Satellites) Pseudo-Range (all bands) < 1.00 cm Carrier Phase (all bands) < 0.01 cm

Ashtech P-Code GPS receivers have been FGCC tested and are capable of performing first order survey (report available upon request).

2-12, Z-Tracking, PNAV and PRISM II are trademarks of Ashtech inc.

Specifications are subject to change without notice

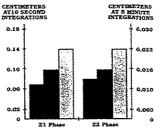


Environm	iental
Waterproof to	o 5 psi
Temperature	Ranges
Receiver/dat	ta Logger
Operating	-20" to +55"C
Storage	-30° to +75°C
Antenna	
Operating	-40° to +65°C
Storage	-55° to +75°℃
Humidity	100%
Weight	
Receiver	8.8 lbs
Antenna	3.75 lbs
Speed (Max)	Does not exceed 1.000 nautical miles-per-hour

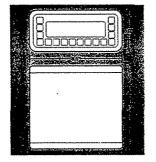
Altitude (Max) Does not exceed 60,000 Ft.

Higher altitude and velocities up to 25,000 nautical miles-per-hour options are available in the U.S. and under validated export license for other countries.

OBSERVABLE RMS IN CENTIMETERS FOR THE 2-12



Dimensions



Standard Features

- · 12 Channel "All-In-View" operation Automatic Switching to Z-Tracking
- when A/S is activated. Full wavelength carrier on L1 and L2
- 21 Watt power consumption (typical)*
- · 10 32 VDC input
- · 2 Power inputs
- · Audible alarm for low power
- Internal RAM data recorder
- 8-Line by 40-character display
- 4 RS-232 ports (115,200 baud max) · Static, rapid static, kinematic,
 - pseudo-kinematic surveys
- . Waypoint navigation
- Real-time data outputs
- 1 PPS timing signal ٠
- · Cold start 2 Minutes to first data
- Warm start <30 Seconds to first data
- I Year warranty

Standard Accessories

- · Precision geodetic antenna
- 10-meter antenna cable
- External power cable
- RS-232 data cable (Z-format)
- Battery and charger Rotatable Tribrach adapter
- High-impact shipping case .

Receiver operating manual (Shipping weight of standard Z-12 package is 48 pounds)

Optional Features

- · External frequency standard input 1 to 20 MHz in 10KHz steps
- Real-time differential GPS
- RTCM format · Expanded internal memory

Optional Accessories

- Survey Tribrach
- Kinematic bipod and pole
- 10. 30 and 60-meter antenna cable
- Expandable to 150 meters w/line amps
- External battery
- Battery charger 110/120 VAC
- PRISM II[™] Software Package PNAV Software Package

*Display off/with LNA

1170 Kifer Road - Sunnyvale, CA 94086 - (408) 524-1400 - Fax (408) 524-1500 Park Place Moscow • 113 Leninski Prospekt • Moscow • Russia • (7502) 256-5400 • Fax (7502) 256-5360 Blenheim Office Park*Lower Road+Long Hanborough-Oxfordshire OX8 8LN+England + 44 993 883 533 + Fax 44 993 883 977



2/94

Figure 2.5. Z-12 technical specifications.

Ashtech Z-12TM GPS Receiver

Full GPS Capability with Anti-Spoofing Turned On

Ashtech's "Dual-Line Digital" Z-12 GPS Receiver sets the standard in GPS receiver performance and technology for precise surveying and navigation applications. This revolutionary new GPS receiver permits uninterrupted use even when Anti-Spoofing (AS) is turned on. When Anti-spoofing is turned on, the Z-12 receiver automatically activates its Z - Tracking™ mode which mitigates the effects of AS. When AS is off, the Z-12 automatically reverts to P-Code mode.

The Z-12 is a new receiver. It is the result of major improvements in all areas of receiver design; RF, digital processing hardware, and substantial algorithmic improvement. As a result, not only does the receiver deliver unmatched performance in "Z" mode, but the performance in "P" mode is world-class (substantially improved over the performance of the pioneering Asthech P-12).

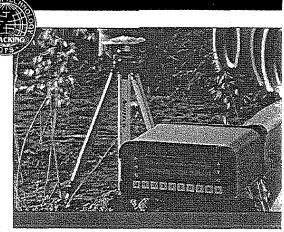
The technological advance represented by this receiver is even more dramatic under Anti-Spoofing (A/S) conditions where the patented Z mode observables enjoy an over 13 dB SNR advantage over their "P-codeless" (cross-correlation) competitors while maintaining the P-mode's freedom from receiver caused systematic errors. Indeed, the receiver measures the same things in both modes: C/A carrier phase and pseudo-range, P1 carrier phase and pseudo-range, P2 carrier phase and pseudo-range, all with full (not 1/2) carrier wavelengths. There are no "glitches" associated with a mode change, no changes in the already negligible systematic errors. For the overwhelming majority of users, the performance of the receiver when A/S is enabled is indistinguishable from the "A/S off" performance.

The Ashtech receiver's patented Z technique is the only available technology that offers an over 13 dB improvement in SNR over cross-correlating receivers along with full wavelength carrier phases on both P-code bands when A/S is enabled.

Mile-a-Minute Surveying

Dual-frequency reception eliminates ionospheric refraction effects, which means medium-to-longer baselines can be measured more accurately. High-quality measurements on both the L1 and L2 bands in the Z-Tracking mode or the P-Code mode also enable significantly shorter station occupation time — this translates into increased productivity for high-precision survey crews. Centimeter-level surveying of baselines of one mile using one minute station occupation'timt@ths been successfully demonstrated in <u>Z-Tracking2Mode</u>!

GPS surveying. The PNAV module is based on an advanced Kalman filter design which allows for nearly instantaneous centimeter-level surveying and navigation for station separations under 10 kilometers.



Seconds vs. Minutes

A 13 dB SNR advantage means a factor of 20 less in integration time for the same observable RMS. Based on actual measurements on real satellites, we need to integrate for 10 seconds to the cross correlation competition's 5 minutes. There are two great advantages to having shorter correlation times for the same SNR:

- The ability to track rapidly varying ionosphere with full observable accuracy. This cannot be accomplished with cross correlating receivers.
- Acquisition transients settle in seconds while the competition has to wait minutes before their A/S observables reach equivalent accuracy.

The ability to derive any useful information at low elevations is critically tied to SNR. When faced with low SNR, the user has a terrible choice: either integrate for such a long time that there is essentially no data at low elevations. or accept huge errors. For all non-classified "A/S on" solutions, the SNR falls off with elevation angle as the square of normal code SNR. That is, if the P mode SNR drops (with elevation angle) by a factor of 4, all civilian A/S techniques yield a drop in the SNR of a factor of 16.

Better Jam Immunity

Because of Ashtech's Dual-Line Digital processing capability, jam immunity is substantially improved over other single bit receivers. The receiver does not lose lock near transmitters or high voltage power lines. The result is higher productivity, robust performance and virtually no restrictions due to an encrypted satellite signal.

PNAV "On-the-Fly" Ambiguity Resolution

Ashtech's newest application software package is called PNAV (for Precision Navigation). This software, combined with dual-frequency data from Z-12 receivers provides a powerful new capability in GPS. PNAV is a precision trajectory package providing post-processed positions and can provide centimeter level accuracy on-the-fly. This capability is especially valuable for creation of robust photogrammetric flight trajectories.

A PNAV survey version which produces vectors for network adjustments is a standard feature of the PRISM IITM software package.



Figure 2.6. Ashtech Z-12tm GPS receiver.

2.3. Application of Kinematic GPS in Photogrammetry

If a GPS antenna is fixed above the camera nodal point in an aircraft (camera antenna), then its position, (see Fig. 2.7.) determined in real time by the kinematic mode, can be used to take aerial photos at predetermined locations. Thus kinematic GPS is used in pinpoint navigation for photogrammetric mapping.

Using differential kinematic GPS, the camera's location (X_0, Y_0, Z_0) can be determined precisely. Thus, in a stereo pair, of the 12 exterior orientation elements, six can be determined by kinematic GPS methods. Five of the exterior elements can be determined by relative orientation and the twelfth element, ω , has to be determined by external ground control.

In a triplet with two photos in the y direction and two photos in the x direction (see Fig. 2.8.), kinematic GPS can be used to determine nine exterior orientation elements and the relative orientation to determine the other nine exterior orientation elements.

In an aircraft, if four antennas are mounted as shown in Fig. 2.7 such that the left wing antenna and the right wing antenna are along the y axis of the aircraft and the camera antenna C and the forward antenna F are along the x axis, then kinematic GPS can be used to determine the locations of these antennas at the time of the exposure. From the location of the antennas, the rotation angles of the aircraft with respect to the ground system (X_G, Y_G, Z_G) can be obtained from:

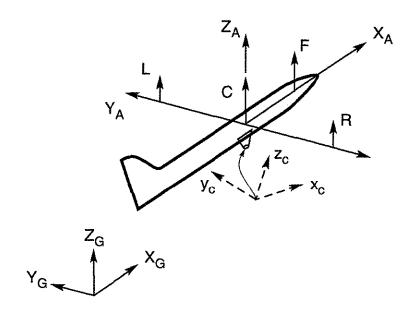
$$Sin \omega_G = (Z_r - Z_1) / LR$$
$$Sin \phi_G = (Z_f - Z_c) / FC$$
$$Sin K_G = (Y_f - Y_c) / FC$$

If R is the rotation matrix which makes the camera axis (x_c,y_c,z_c) parallel to the aircraft axis (x_A,y_A,z_A) , then the rotation angles of the camera is given by:

$$A = R A' R^T$$

where

$$A' = R K_G * R\phi_G * R\omega_G$$
 and $A = R K_c * R\phi_c * R\omega_c$



•

.

Figure 2.7. Multiantenna locations.

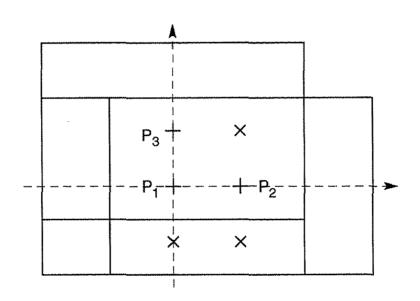


Figure 2.8. Triplet

Thus in an aerial photo all of the exterior orientation can be determined by kinematic GPS provided the parameters of the matrix R are determined by calibration. No ground control is required for rectification, stereo plotting, and orthophoto production.

3. ANALYSIS OF FIRST TEST

Figure 3.1 shows the Cessna 335 aircraft used in this project. An L_1/L_2 GPS antenna was mounted about 1.164 meters above the nodal point of the camera. The camera was located on the center fuselage floor with its lens 0.775 meters above the taxiway. The second antenna (L_1) was mounted on the left wingtip aft of the wingtip fuel tank. The wingtip L_1 antenna was approximately 1.62 meters above the taxiway and 5.12 meters from the camera antenna (see Fig. 3.2).

A P-12 GPS receiver was set over the base station, surdex, and two P-12 GPS receivers were placed inside the aircraft, connected to the camera and wing antennas (see Fig. 3.3). The GPS receivers recorded the position at every one-second interval. Photographs were taken over the test range in St. Louis at flying heights of 1,500 feet and 3,000 feet (See flight plan, Fig. 3.4). The GPS was used to tie the control points in the test range to the National Geodetic Network, enabling the control point coordinates to be transformed to the NAD83 system.

The GPS data were processed using the PNAV software. This software has the unique capability of precisely computing relative locations between two GPS stations when one or both receivers are moving. Tables 3.1 and 3.2 show the camera and wing antenna locations at the exposure times of the camera. Graph 3.1 shows the Z difference between the camera and wing antennas, indicating that the Z difference in the low flight was not reliable because the wing antenna is an L₁ antenna and was sometime tracking \leq four satellites (see Table 3.2). An L₁/L₂ antenna enables the P code to be accessed and seven or more satellites enable the resolution of the ambiguity during the flight.

High and low flight photographs were observed using the wild stereocomparator. The data were initially processed by Albany software and then by Calib. Calib is a special software which can simultaneously calibrate the camera as well as constrain both exterior orientation elements and ground control.

Graph 3.2 shows the omega, w, angle from photogrammetry and GPS. The difference between them may be due to such factors as (1) deflection of the wing due to lift of the aircraft, (2) initial angular difference between the film plane of the camera and the plane of the aircraft wing, and (3) initial angular difference between the x axis of the camera and the main axis of the aircraft.

The difference arising out of the first factor may be difficult to model, but this may be overcome by using a filtering technique. Graph 3.3 shows the original and filtered height differences between the camera and wing antennas for the high-altitude flight. It appears that filtering eliminates the effect of the deflection to a first order. The second and third causes could be modeled by the equation



Figure 3.1. Aircraft used in tests.

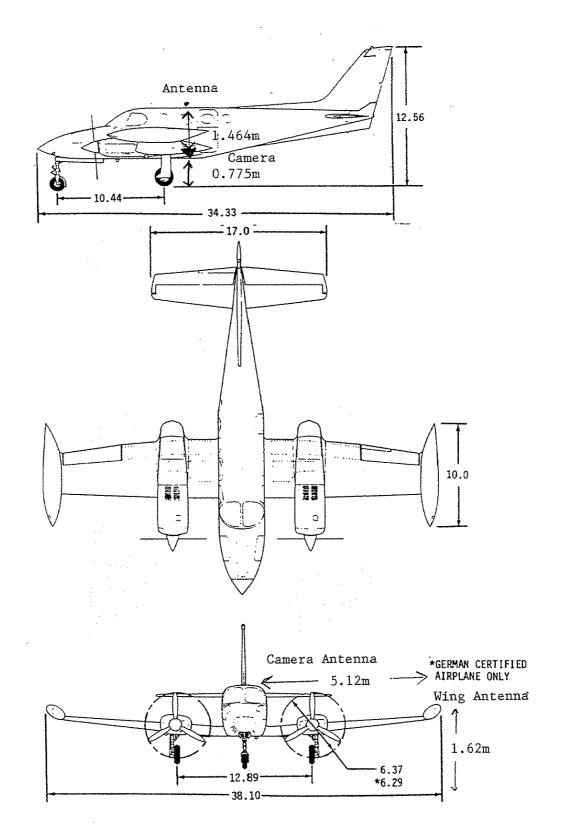


Figure 3.2. Aircraft dimension.

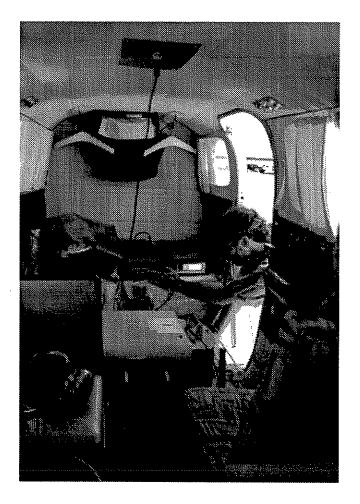
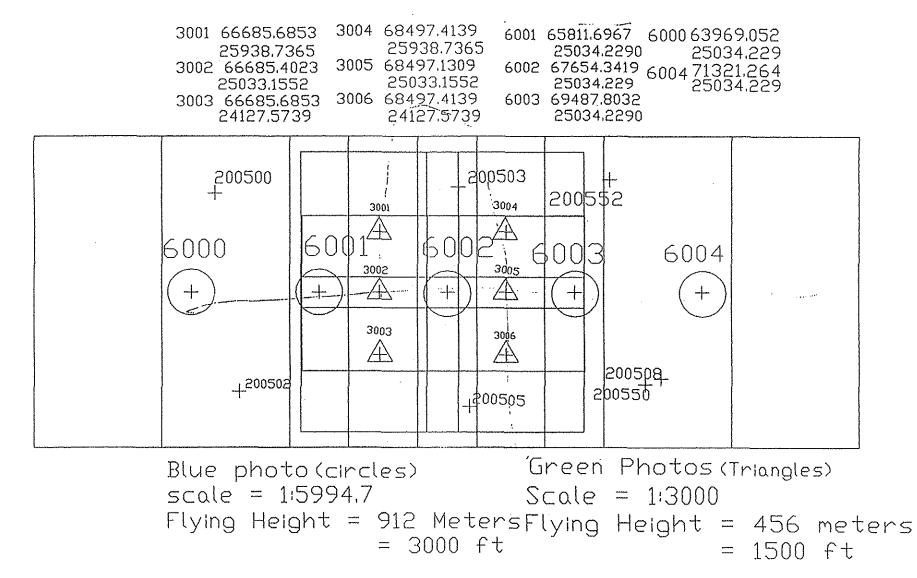


Figure 3.3. Camera and receiver inside aircraft.



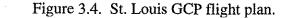
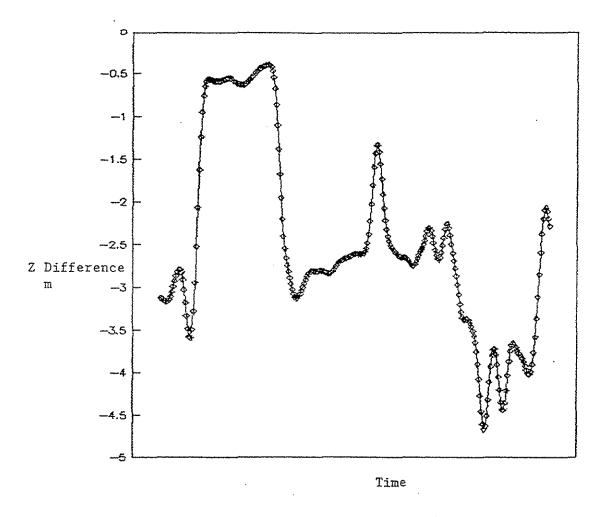


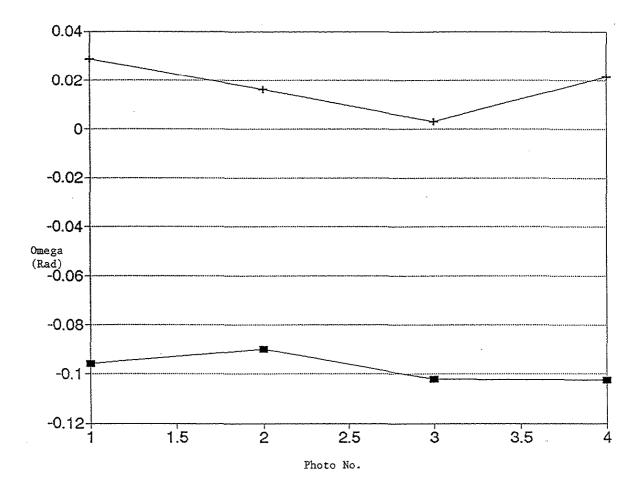
Table 3.1. Camera antenna position.

Table 3.2. Wing antenna position.

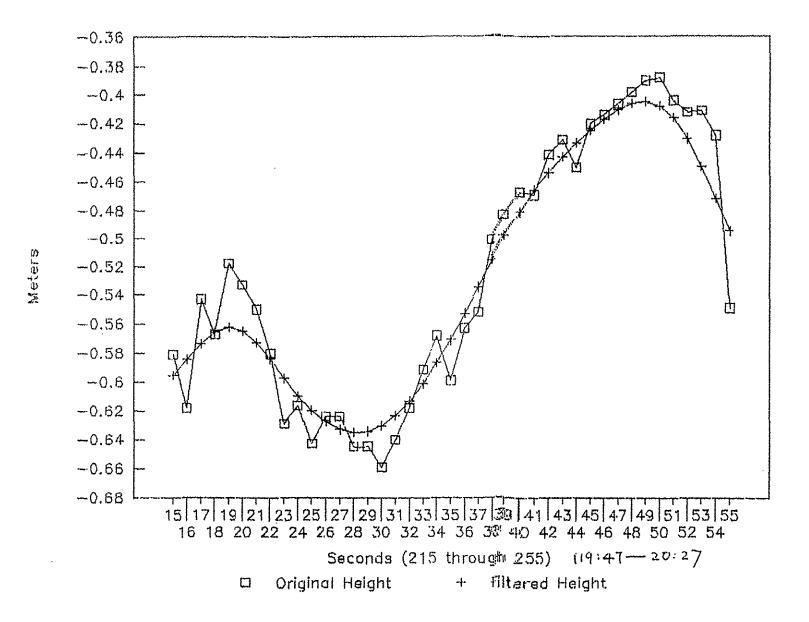
Ashtech, Inc.	GPPS-2	Pro	gram:		PPDIFF-PNAV		Version: 1.0.	00
	Wed Jun 30 10:33	3:13	1993		Differentia]	lly	Corrected: Y	
SITE MM/DD/YY	HH:MM:SS	SVs	PDOP		LATITUDE		LONGITUDE	нт
_WIN 06/18/93	18:16:23.585673	4	3.7	Ņ	1 38.60819815	W	90.53162880	1069.5884
_WIN 06/18/93	18:16:29.828086	4	3.7	N	38.60845906	W	90.52560963	1069.1986
_WIN 06/18/93	18:16:36.017254	5	1.8	Ŋ	1 38.60856843	W	90.51962076	1070.5178
_WIN 06/18/93	18:16:42.185334	5	1.8	N	1 38.60846829	W	90.51365598	1069.5413
_WIN 06/18/93	18:16:48.337325	5	1.8	N	38.60817890	W	90.50772654	1070.2871
_WIN 06/18/93	18:19:45.655955	6	2.4	N	38.60432737	W	90 53802231	1072.2670
_WIN 06/18/93	18:19:51.969862	7	1.5	N	38.60425058	W	90.53187378	1070.0468
_WIN 06/18/93	18:19:58.331937	7	1.5	N	38.60420161	W	90.52563515	1063.3098
_WIN 06/18/93	18:20:04.€69017	7	1.5	N	38.60424303	W	90.51944541	1068.0905
_WIN 06/18/93	18:20:10.985137	7	1.5	N	38.60428885	W	90.51330098	1671.4706
_WIN 06/18/93	18:20:17.316511	7	1.5	N	38.60423532	W	90.50712638	1069.8176
_WIN 06/18/93	18:27:28.610186	5	2.4	N	38.60962970	W	90.53928599	613.0895
_WIN 06/18/93	18:27:32.032063	5	2.4	N	38.60729634	W	90.53928820	607.3155
_WIN 06/18/93	18:27:35.487433	5	2.4	N	38.60492668	W	90.53926395	605.4586
_WIN 06/18/93	18:27:38.936745	5	2.4	N	38.60256367	W	90.53925329	602.6547
_WIN 06/18/93	18:27:42.375848	5	2.4	N	38.60022195	W	90 53921353	603.8206
_WIN 06/18/93	18:27:45.815082	5	2.4	N	38.59789286	W	90.53913753	608.1482
_WIN 06/18/93	18:27:49.275442	5	2.4	N	38.59555054	W	90.53905437	613.1926
_WIN 06/18/93	18:30:31.346137	4	20.8	N	38.61142047	W	90.53322753	634.8411
_WIN 06/18/93	18:30:34.777854	5	2.3	N	38.60825889	W	90.53297292	610.8343
_WIN 06/18/93	18:30:38.224967	5	2.3	N	38.60594821	W	90.53303491	609.5768
	18:30:41.670012	5	2.3	N	38.60362754	W	90.53305199	606.3610
_WIN 06/18/93	18:30:45.123006	5	2.3.	N	38.60128920	W	90.53303954	608.9579
_WIN 06/18/93	18:30:48.589834	5	2.3	N	38.59894026	Ŵ	90.53301455	607.4602
_WIN 06/18/93	18:30:52.051058	5	2.3	N	38.59658793	W	90.53297397	608.0400
_WIN 06/18/93	18:30:55.489186	5	2.3	Ν	38.59424812	W	90.53291786	609.0595
_WIN 06/18/93	18:35:46.817115	0	17.4	N	38.61136476	W	90.52605769	638.4910
_WIN 06/18/93	18:35:50.267464	4	35.0	N	38.60910617	W	90.52599801	644.4634
_WIN 06/18/93	18:35:53.708846	4	35.3	Ν	38.60690914	W	90.52592090	656.4001
_WIN 06/18/93	18:35:57.143818	4	35.6	Ν	38.60472721	Ŵ	90.52584555	660.5251
_WIN 06/18/93	18:36:00.598300	5	2.1	Ν	38.60254253	Ŵ	90.52575816	660.2306
_WIN 06/18/93	18:36:04.050185	5	2.1	Ν	38.60039630	W	90.52567464	651.9298
_WIN 06/18/93	18:36:07.483497	5	2.1	N	38.59826567	W	90.52562565	638.0428
_WIN 06/18/93	18:38:53.378486	6	1.8	N	38.61022467	W	90.51873878	609.3553
-	18:38:56.831284	6	1.8		38.60800168	W	90.51877199	609.6206
_WIN 06/18/93	18:39:00.280785	6	1.8		38.60578509	W	90.51879142	614.0297
_WIN 06/18/93	18:39:03.740073	6	1.8	N	38.60355435	W	90.51379511	616.1731
_WIN 06/18/93	18:39:07.180434	6	1.8	N	38.60131907	W	90.51880473	611.2980
	18:39:10.628531	6	1.8	N	38.59906437	W	90.51882701	606.8798
_WIN 06/18/93	18:39:14.072252	6	1.8	N	38.59681721	W	90.51887242	606.2544



Graph 3.1. Camera-wing Z difference vs. time.



Graph 3.2. St. Louis test 1 (photogrammetry) vs. GPS.



Graph 3.3. Original and filtered height.

$$w_p = w_o + w_G(A \operatorname{task} + B \operatorname{sink})$$

where

 w_p = camera's omega rotation from photogrammetry

 w_G = aircraft's omega rotation from GPS

k = aircraft's kappa rotation from GPS

A, B, $w_0 = constants$

Table 3.3 gives the results of the computation for A, B, and w_0 for the high flight, showing that the model using the filtered values agrees with the photogrammetric value within 0.0003 radians.

As a first-order correction, this model is satisfactory, considering the fact that error exists as a result of using L_1 wing antenna and the error also exists in w determined by photogrammetry. These results show that determining w by GPS is feasible.

Photo #	0	megap	К	appa	C)mega _g (Filtere	d) Omega _g	(Original)		
	()	rad)	(rad)			(rad)	(ra	d)		
6	-0.0)28599	-0.1	10516		-0.085093	0.0	5494		
7	-0.0)16348	-0.0	69752		-0.094603	0.07	0.07373		
8	-0.0)31913	-0.0	68699		-0.105448	0.05	5692		
9	-0.0	021480	-0.0	62829		-0.101363	0.07	7315		
			C	omparis	sioi	n of Solution				
	Solution with 6, 7 & 8 Photos Check with Photo 9							th Photo 9		
Source for										
Omega	1	Omega ₀		A		В	Phic	Phi _c (calculated)		
Filtered	Filtered 0.126903 0.		0.972	761	-7.803094	-0.021480	-0.021166			
	Error: 1.1%				1.1%					
Origina]	-0.084	026	0.75884		-2.308862	-0.021480	-0.018022		
					Error:	16.1%				

Table 3.3. Comparison of ω by GPS and photogrammetry.

4. ANALYSIS OF SECOND TEST

The objective of this test was to evaluate the use of GPS in obtaining pinpoint aerial photographs. In this test, the aircraft is equipped with an L_1 antenna over the cockpit of the aircraft and C/A code GPS receiver with navigation software. The navigation software triggers the camera when the predetermined location agrees with the limit set for the aircraft's position as determined by the C/A code GPS receiver.

Figure 4.1 shows the proposed flight plan over ISU campus and the predetermined exposure station coordinates. Two flights, one at 1,500-feet flying height (1 inch = 250 feet scale photographs) and the other at 2,250-feet flying height, were proposed.

Typical specifications for aerial photography are that the exposure station is within 1/2 inch on the photograph of the proposed station and the actual flying height is within 5% of the proposed flying height. Thus, a tolerance of \pm 50 meters was set on the GPS navigation system. Due to the uncertainty of the height determination by the C/A code receiver, only latitude and longitude by GPS navigation was entered into the computer on board and the height was determined by the onboard aneroid barometer. Table 4.1 shows the coordinates of the exposure station used by the onboard navigation system.

A block of photographs were observed using the Wild Stereo Comparator, and the block adjustment was performed by the Albany software. Table 4.2 shows the difference between the exposure coordinates that were proposed and then obtained by photogrammetry (Albany software).

Figure 4.2 shows the proposed flight lines, the flight lines from the layout diagram prepared using the exposed photographs, and those using the exposure coordinates determined by Albany.

The flight lines in Figure 4.2 and the standard error of the difference in coordinates of ± 27 meters indicate that pinpoint navigation is satisfactory. Even though the proposed coordinates (Fig. 4.1) and the navigation coordinates (Table 4.1) are almost identical, they are different from the Albany with a maximum difference of 42 meters and a standard error of ± 27 meters. This shows that the navigation software's performance is satisfactory but the position determined by C/A code GPS receiver is off by ± 27 meters. The standard error of ± 27 meters is within the specification allowed (1/2 inch x 250 feet = 125 feet $\simeq 40$ meters). This error is expected of the C/A code GPS receiver. The accuracy can be improved either by using differential real-time C/A code receiver or a P/code receiver.

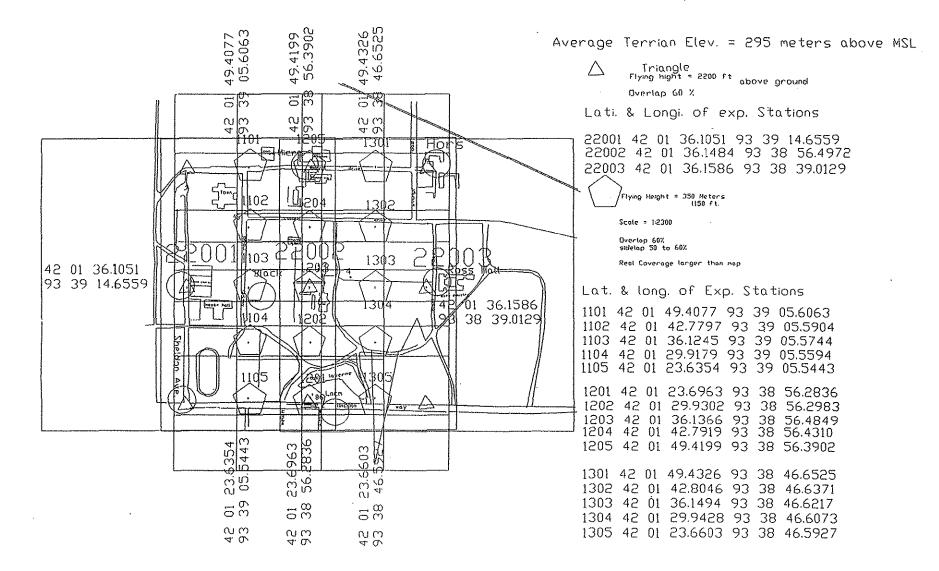


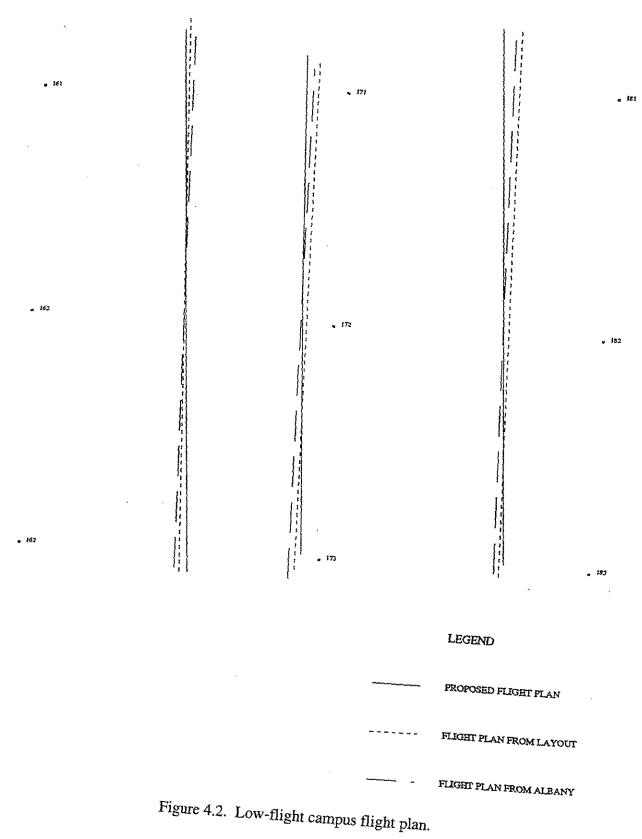
Figure 4.1. Campus flight plan diagram.

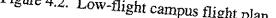
Table 4.1. Camera location from navigation software.

00002	930823	184947	ISU	CAMPUS	01		N42.090874	W093.516394	04776T	266
00003	930823	185626	ISU	CAMPUS	01	001	N42.030125	W093.651481	02140T	181
00004	930823	185628	ISU	CAMPUS	01	002	N42.028285	W093.651539	02140T	181
00005	930823	185631	ISU	CAMPUS	01	003	N42.026436	W093.651622	02244T	182
00006	930823	185634	ISU	CAMPUS	01	004	N42.024411	W093.651697	02244T	182
00007	930823	185636	ISU	CAMPUS	01	005	N42.022767	W093.651726	02244T	181
80000	930823	185931	ISU	CAMPUS	02	001	N42.023495	W093.648868	02282T	360
00009	930823	185933	ISU	CAMPUS	02	002	N42.025370	W093.648854	02282T	360
00010	930823	185936	ISU	CAMPUS	02	003	N42.027245	W093.648850	02282T	360
00011	930823	185938	ISU	CAMPUS	02	004	N42.029121	W093.648887	02282T	359
00012	930823	185941	ISU	CAMPUS	02	005	N42.030993	W093.648970	02282T	358
00013	930823	190247	ISU	CAMPUS	03	001	N42.030144	W093.646515	02282T	182
00014	930823	190250	ISU	CAMPUS	03	002	N42.028216	W093.646553	02282T	181
00015	930823	190253	ISU	CAMPUS	03	003	N42.026466	W093.646537	02282T	180
00016	930823	190255	ISU	CAMPUS	0.3	004	N42.024551	W093.646484	02176T	179
00017	930823	190258	ISU	CAMPUS	03	005	N42.022814	W093.646441	02176T	179
00018	930823	190730	ISU	CAMPUS	04	001	N42.026562	W093.653621	03237T	090
00019	930823	190734	ISU	CAMPUS	04	002	N42.026575	W093.648833	03237T	090
00020	930823	190738	ISU	CAMPUS	04	003	N42.026619	W093.644065	03344T	089
00021	930823	190743	ISU	CAMPUS	04	004	N42.026645	W093.639295	03344T	090

Low Flight	Diff. in Easting	Diff. in Northing	Diff. in Elev.						
	mts	mts	mts						
01	6.638	3.582	-19.400						
02	12.840	1.471	-16.908						
03	12.416	-1.556	-14.499						
04	24.375	36.684	-10.142						
05	27.474	42.451	-5.888						
Mean	16.747	16.52	-13.364						
Stand. Er	ror: 23.5 m								
High Flight									
06	7.239	9.335	-27.730						
07	32.522	7.705	-23.320						
08	41.224	0.971	-22.300						
Mean	26.998	6.00	-24.4						
Stand. Error: 27.6 m									

Table 4.2. Comparison of GPS navigation and photogrammetry.





5. ANALYSIS OF THIRD TEST

The objective of this test is to evaluate the feasibility of using four antennas on the aircraft, the accuracy of the 3DF receivers, and the reliability of the L_1/L_2 antenna on the wing. This test was done over the test range in St. Louis in October 1993.

After careful study of the Cessna 335 aircraft, it was decided to have one antenna on the left wing, one on the right wing and one on the tail in addition to one above the camera. Figure 5.1 shows the aircraft and the location of the antennas. The camera and right wing were installed with L_1/L_2 antennas and connected to the P12 receivers. The left wing and tail had L_1 antennas; these two were connected to the 3DF receivers. Using a signal splitter, the camera antenna was also connected to the 3DF receivers.

Using the same flight plan as in test one (see Fig. 3.3) aerial photographs were taken over the St. Louis test range at 1,500-feet and 3,000-feet flying heights. Photographs were observed using the Wild stereo comparator, and the block adjustment were done by both Albany and Calib softwares. GPS data were processed by PNAV and 3DF software.

Tables 5.1, 5.2, and 5.3 give the position of the camera, the right wing, and the tail at the camera exposure time as determined by PNAV and 3DF software. These tables indicate that both camera and right wing antennas track seven to eight satellites continuously, while the tail antenna drops to four satellites. These tables also indicate that positions are determined to ± 0.1 millimeter accuracy. Graphs 5.1 and 5.2 show the flight path of the camera and the tail antenna are not reliable. This may be due either to the L₁ antenna or its location on the tail of the aircraft. Because the 3DF software relies on the tail antenna, we were not able to get reliable values on the left wing. Table 5.4 shows the rotation angles computed from the camera, the tail, and right wing antenna locations. Angles kappa and phi depends on the location of the tail antenna and therefore may not be reliable.

Graphs 5.3 and 5.4 show the comparison of omega (ω) rotation angle by Albany (photogrammetry), by Yao (rotation by GPS), and by Ken (rotation by GPS from the initial position) for high and low flights, respectively. As in the first test flight, the graphs indicate a direct relationship between w rotation obtained by GPS and by photogrammetry.

In order to correctly model the relationship between w rotation by GPS and photogrammetry, various analyses were done. Graph 5.5 shows the true difference in height between the right wing and camera and the filtered height difference, indicating that there is flexing of the wing due to air lift, etc., which is independent of the rotation. Graph 5.6 shows the change in length between the camera antenna and the right wing antenna for the high flight. From Figure 5.2, it appears that the change in length is strictly due to flexing of the wing. In order to

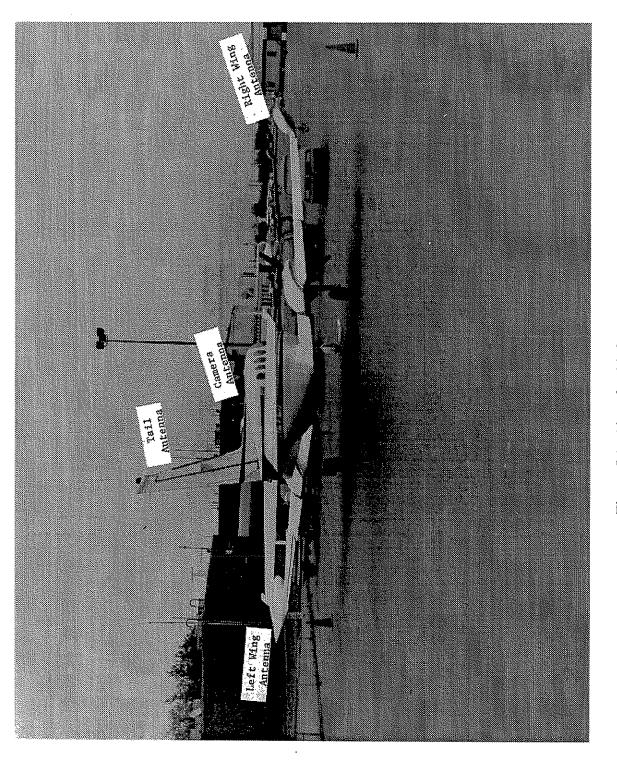


Figure 5.1. Aircraft with four antennas.

Table 5.1. Camera antenna position.

	: PPDIFF-PNAV Version: 1.0.00	
Sun Jan 16 22:54:52 1994	Differentially Corrected: Y	
	S PDOP LATITUDE LONGITUDE HI	RMS FLAG
?CAM 10/30/93 14:50:17.320722 5	1.9 N 38.66307853 W 90.64520684 143.7205	0.154 1
?CAM 10/30/93 14:55:49.666998 5	2.0 N 38.66260023 W 90.63527466 142.3183	0.155 1
?CAM 10/30/93 14:56:05.304581 5	2.0 N 38.66260018 W 90.63527463 142.3178	0.155 1
?CAM 10/30/93 15:03:39.986363 7	1.3 N 38.60407215 W 90.54144354 957.6226	0.155 1
?CAM 10/30/93 15:03:45.212922 7	1.3 N 38.60404261 W 90,53616305 962.3996	0.157 1
?CAM 10/30/93 15:03:50.476001 7	1.3 N 38.60401539 W 90.53085989 960.9707	0.158 1
?CAM 10/30/93 15:03:55,714706 7	1.3 N 38.60398297 W 90.52557454 960.3949	0.158 1
?CAM 10/30/93 15:04:00,946909 7	1.3 N 38.60390759 W 90.52029405 964.4583	0.159 1
?CAM 10/30/93 15:04:06.193357 7	1.3 N 38.60378094 W 90.51499654 967.4111	0.160 1
?CAM 10/30/93 15:04:11.487352 7	1.3 N 38.60364861 W 90.50964996 965.0516	0.175 1
?CAM 10/30/93 15:13:12.118887 8	1.2 N 38.59498291 W 90.51962517 657.0267	0.071 1
?CAM 10/30/93 15:13:16.102004 8	1.2 N 38.59735429 W 90.51951564 660.6232	0.071 1
?CAM 10/30/93 15:13:20.574425 8	1.2 N 38.60002335 W 90,51943985 663.6988	0.070 1
?CAM 10/30/93 15:13:25.036935 8	1.2 N 38.60272699 W 90.51948142 662.7284	0.076 1
?CAM 10/30/93 15:13:29.468310 8	1.2 N 38.60545617 W 90.51957245 663.9462	0.070 1
?CAM 10/30/93 15:13:33,929109 8	1.2 N 38.60823181 W 90.51962691 668.7381	0.070 1
?CAM 10/30/93 15:13:38,408214 8	1.2 N 38.61104584 W 90.51960587 665.5211	0.070 1
?CAM 10/30/93 15:15:49.963353 7	1.3 N 38.61090076 W 90.53020496 662.6389	0.070 1
?CAM 10/30/93 15:15:53,159791 7	1.3 N 38.60848756 W 90.53023149 667.4687	0.070 1
?CAM 10/30/93 15:15:56,580263 7	1.3 N 38.60590356 W 90.53029259 665.5371	0.070 1
?CAM 10/30/93 15:16:00.039792 7	1.3 N 38.60328000 W 90,53038132 666.2825	0.071 1
?CAM 10/30/93 15:16:03.551927 8	1.2 N 38,60060243 W 90,53047462 667,5936	0.069 1
?CAM 10/30/93 15:16:07.013033 8	1.2 N 38.59795525 W 90.53053791 670.9608	0.068 1
?CAM 10/30/93 15:16:10.470674 8	1.2 N 38.59530605 W 90.53057048 674.5688	0.069 1
?CAM 10/30/93 15:19:22.125536 7	1.6 N 38,59651863 W 90,52506928 651,3185	0.082 1
?CAM 10/30/93 15:19:26.594989 7	1.6 N 38.59922830 W 90.52488740 656.0809	0.082 1
?CAM 10/30/93 15:19:31.052658 7	1.6 N 38.60195879 W 90.52472534 655.8816	0.081 1
2CAM 10/30/93 15:19:35.503228 7	1.6 N 38.60470340 W 90.52463179 655.7915	0.081 1
?CAM 10/30/93 15:19:39.761709 7	1.6 N 38.60732770 W 90.52460261 653.9743	0.081 1
?CAM 10/30/93 15:19:43.924864 7	1.6 N 38.60989157 W 90.52462232 654.4512	0.080 1
?CAM 10/30/93 15:20:02.309311 7	1.6 N 38.62111373 W 90.52470759 652.0795	0.078 1
?CAM 10/30/93 15:20:05.058039 7	1.6 N 38.62281098 W 90.52473964 650.5285	0.078 1
2CAM 10/30/93 15:20:03:0600035 7	1.6 N 38.62479519 W 90.52476504 600.5265	0.078 1
?CAM 10/30/93 15:20:00.201000 /	1.6 N 38.62650210 W 90.52483807 643.9597	0.078 1
?CAM 10/30/93 15:20:11:0008092 7	1.6 N 38.62962522 W 90.52493763 641.9240	0.078 1
2CAM 10/30/93 15:20:16:000092 7 2CAM 10/30/93 15:20:19:663155 7	1.6 N 38.63189334 W 90.52500538 643.3966	0.091 1
?CAM 10/30/93 15:20:19:865155 7 ?CAM 10/30/93 15:20:22:400918 7	1.6 N 38.63357601 W 90.52506064 642.3124	0.077 1
?CAM 10/30/93 15:20:22.400916 7 ?CAM 10/30/93 15:20:26.038116 7	1.6 N 38.63578306 W 90.52506064 642.5124	0.077 1
	1.6 N 38.63769403 W 90.52532560 639.6420	
?CAM 10/30/93 15:20:29.240071 7	1.6 N 38.63931093 W 90.52551634 634.2945	0.076 1
?CAM 10/30/93 15:20:31.981183 7		
rommi 10/30/33 15:21:06.106349 7	1.6 N 38.65792614 W 90.53817315 533.2000	0.074 1

.

Table 5.2. Right wing antenna position.

.

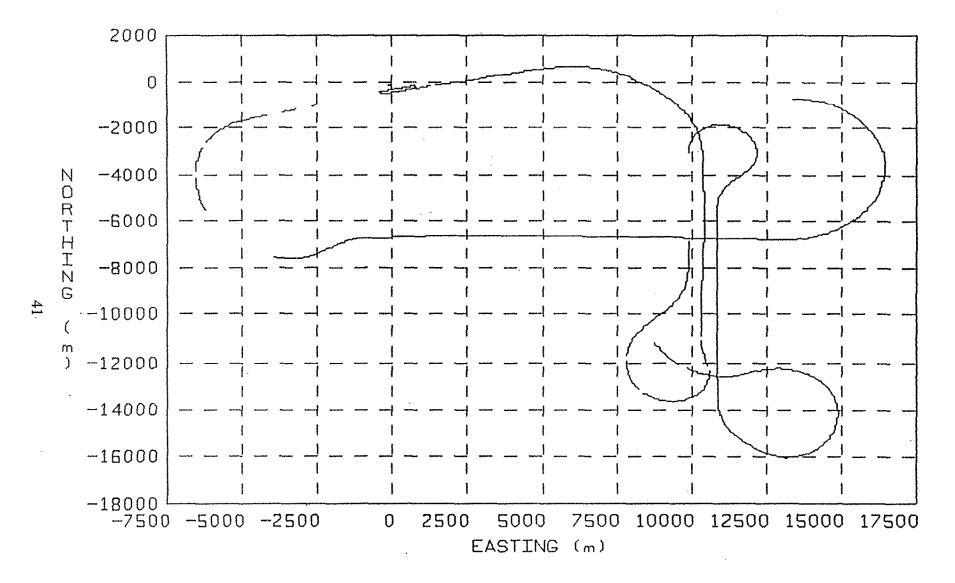
Ashtech, Inc. GPPS-2 Program: PPDIFF-PNAV Version: 1.0.00		
Sun Jan 16 22:51:37 1994 Differentially Corrected: Y		
SITE MW/DD/YY HH:MM:SS SVs PDOP LATITUDE LONGITUDE HI	RMS	FLAG
2CAM 10/30/93 14:50:17.320722 5 1.5 N 38.66303354 W 90.64519402 143.270		1
2CAM 10/30/93 14:55:49.666998 5 1.5 N 38.66259613 W 90.63534171 141.901		1
2CAM 10/30/93 14:56:05.304581 5 1.5 N 38.66259556 W 90.63534132 141.852		1
?CAM 10/30/93 15:03:39.986363 7 1.3 N 38.60402378 W 90.54143370 956.545		1
?CAM 10/30/93 15:03:45.212922 7 1.3 N 38.60399412 W 90.53615359 961.403	6 0.126	1
?CAM 10/30/93 15:03:50.476001 7 1.3 N 38.60396680 W 90.53085106 959.997	3 0.127	1
?CAM 10/30/93 15:03:55.714706 7 1.3 N 38.60393423 W 90.52556770 959.386	5 0.127	1
?CAM 10/30/93 15:04:00.946909 7 1.3 N 38.60385892 W 90.52028613 963,422	3 0.128	1
?CAM 10/30/93 15:04:06.193357 7 1.3 N 38.60373231 W 90.51498788 966.377	7 0.128	1
2CAM 10/30/93 15:04:11.487352 7 1.3 N 38.60360132 W 90.50964004 964.075		1.
2CAM 10/30/93 15:13:12.118887 8 1.2 N 38.59498180 W 90.51956368 657.302		1
7CAM 10/30/93 15:13:16.102004 8 1.2 N 38.59735518 W 90.51945409 661.004	3 0.108	1.
?CAM 10/30/93 15:13:20.574425 8 1.2 N 38.60002675 W 90.51937849 664.132		1
7CAM 10/30/93 15:13:25.036935 8 1.2 N 38.60272975 W 90.51941991 663.182		1
?CAM 10/30/93 15:13:29.468310 8 1.2 N 38.60545867 W 90.51951109 664.204		1
7CAM 10/30/93 15:13:33.929109 8 1.2 N 38.60823156 W 90.51956515 669.006		1.
?CAM 10/30/93 15:13:38.408214 8 1.2 N 38.61104514 W 90.51954394 665.810		1
?CAM 10/30/93 15:15:49.963353 8 1.2 N 38.61090142 W 90.53026876 662.455		1
?CAM 10/30/93 15:15:53.159791 8 1.2 N 38.60848782 W 90.53029524 667.254		1
?CAM 10/30/93 15:15:56.580263 8 1.2 N 38.60590505 W 90.53035635 665.4179		1
2CAM 10/30/93 15:16:00.039792 8 1.2 N 38.60328089 W 90.53044516 666.1730		1
?CAM 10/30/93 15:16:03.551927 8 1.2 N 38.60060212 W 90.53053854 667.5078		•
2CAM 10/30/93 15:16:07.013033 8 1.2 N 38.59795398 W 90.53060198 670.9756		1
2CAM 10/30/93 15:16:10.470674 8 1.2 N 38:59530526 W 90.53063438 674.439		1
2CAM 10/30/93 15:19:22.125536 7 1.6 N 38.59651770 W 90.52500416 651.2097		1
CAM 10/30/93 15:19:26.594989 7 1.6 N 38.59922751 W 90.52482232 655.9498		1
7CAM 10/30/93 15:19:31.052658 7 1.6 N 38.60196005 W 90.52466025 655.8642		1
7CAM 10/30/93 15:19:35:503228 7 1.6 N 38:60470494 W 90:52456670 655:7934		1
2CAM 10/30/93 15:19:39.761709 7 1.6 N 38:60733028 W 90:52453759 654.0230		1
2CAM 10/30/93 15:19:35.761709 7 1.6 N 38:60989320 W 90:52455735 654.3655		1
*CAM 10/30/93 15:19,43.924604 7 1.6 N 36:00369520 W 90:52455755 654.565 *CAM 10/30/93 15:20:02.309311 7 1.6 N 38:62111606 W 90:52464262 652.0422		1
2CAM 10/30/93 15:20:02:309311 7 1.6 N 38:62281314 W 90:52464262 652:0422 2CAM 10/30/93 15:20:05:058039 7 1.6 N 38:62281314 W 90:52467469 650:4391		1
2CAM 10/30/93 15:20:03:030039 7 1.6 N 38:62479704 W 90:52407469 630:4351		1
2CAM 10/30/93 15:20:11.005684 7 1.6 N 38.62650488 W 90.52477336 643.7568	*****	1
2CAM 10/30/93 15:20:16.008092 7 1.6 N 38.62962755 W 90.52487276 641.7796		1
?CAM 10/30/93 15:20:19.663155 7 1.6 N 38.63189494 W 90.52494022 643.4270		1
2CAM 10/30/93 15:20:22,400918 7 1.6 N 38.63357899 W 90.52499567 642.3846		1
2CAM 10/30/93 15:20:26.038116 7 1.6 N 38.63578597 W 90.52510396 643.3714		1
?CAM 10/30/93 15:20:29.240071 7 1.6 N 38.63769856 W 90.52526071 639.8971		1
2CAM 10/30/93 15:20:31.981183 7 1.6 N 38.63931656 W 90.52545167 634.7225		1
?CAM 10/30/93 15:21:08.106349 7 1.6 N 38.65796101 W 90.53812837 533.3244	0.172	1

Table 5.3. Tail antenna position.

-								
Ashtech, Inc.	GPPS-2	Pro	ram:		PPDIFF-PNAV		Version: 1.0.	00
	Wed Feb 09 13:3				Differential	lv	Corrected: Y	
SITE MM/DD/YY			PDOP		LATITUDE		LONGITUDE	HI
?CAM 10/30/93		6	1.5	N	38.66306793	W	90.64525918	145.5637
?CAM 10/30/93		5	2.1		38.66264597	W	90.63526991	144.0823
		5	2.1		38.66264588	W	90.63526979	144.0913
?CAM 10/30/93		5	2.4		38.60407224	W	90.54149706	956.6942
?CAM 10/30/93		5	2.4		38.60404302	W	90.53621641	961.5671
?CAM 10/30/93		5	2.4		38.60401622	W	90.53091329	960.1770
?CAM 10/30/93		5	2.4		38.60398511	Ŵ	90.52562833	959.5804
?CAM 10/30/93		5	2.4		38.60390879	W	90.52034780	963.6732
?CAM 10/30/93	15:04:06.193357	5	2.4		38.60378146	W	90.51505022	966.6614
?CAM 10/30/93		5	2.4		38.60364939	W	90.50970366	964.3119
?CAM 10/30/93		5	2.7		38.59492882	W	90,51962861	659.8728
	15:13:16.102004	5	2.7		38.59730030	W	90.51951694	663.5087
	15:13:20.574425	5	2.7		38,59996995	W	90.51943823	666.7211
	15:13:25.036935	5	2.7		38.60267350	W	90.51948071	665.7725
	15:13:29.468310	5	2.7		38.60540259	W	90.51957164	666.9113
	15:13:33.929109	5	2.7		38.60817814	w	90.51962947	671.7459
	15:13:38.408214	5	2.7		38.61099212	w	90.51960922	668.5159
	15:15:49.963353	4	3.2	-	38.61090378	W	90.53041827	667.4687
	15:15:49.983353	4	3.2		38.60850584	W	90.53041827	
?CAM 10/30/93		4 4			38.60593984	W		667.4221
	15:16:00.039792	4 4	3.2 3.5		38.60334407	W	90.53038955	667.3723
		4 4		-			90.53037566	667.3558
	15:16:03.551927		3.5		38.60066628	W	90.53047026	668.5748
	15:16:07.013033	4	3.5		38.59801908	W	90.53053447	671.9565
	15:16:10.470674	4	3.5		38.59537007	W	90.53056678	675.5016
?CAM 10/30/93		6	1.7		38.59646598	W	90.52506780	653.9370
	15:19:26.594989	6	1.7		38.59917573	W	90.52488567	658.7526
		6	1.7		38.60190623	W	90.52472146	658.5154
	15:19:35.503228	6	1.7		38.60465082	W	90.52462763	658.4139
	15:19:39.761709	6	1.7		38.60727520	W	90.52459728	656.6147
	15:19:43.924864	6	1.7		38.60983888	W	90.52461793	657.0066
?CAM 10/30/93		6	1.7		38.62106129	W	90.52470231	654.7988
?CAM 10/30/93		6	1.7		38.62275859	W	90.52473445	653.2670
?CAM 10/30/93		6	1.7		38.62474295	W	90.52478219	650.7481
?CAM 10/30/93		6	1.7		38.62644997	W	90.52483182	् 646.761 3
?CAM 10/30/93		6	1.7	N	38.62957284	W	90.52493227	644.6209
?CAM 10/30/93		6	1.7		38.63184083	W	90.52500111	646.0876
?CAM 10/30/93	15:20:22.400918	6	1.7		38.63352360	W	90.52505489	644 .9 893
	15:20:26.038116	6	1.7		38.63573064	W	90.52516409	645.6935
?CAM 10/30/93	15:20:29.240071	6	1.7	N	38.63764184	W	90.52531848	642.3139
	15:20:31.981183	6	1.7	N	38.63925902	W	90.52550826	637.0666
?CAM 10/30/93	15:21:08.106349	5	3.0	N	38.65788779	W	90.53812888	535.9311

7500 10000 12500 15000 17500 I 1 Į l 1 I 1 ł ļ ł 1 Ū. 1 I 1 I 1 1 (m 1 I EASTING 5000 1 2500 \star I 1 0 l 1 1 1 1 1 1 -2500 I | 1 1 1 ł -5000 1] I 1 1 -18000 --40002000 0 -2000 -14000-16000-8000 -12000 -6000 -10000د ع ZOKHIHZU

Graph 5.1. Flight path of Surdex aircraft.



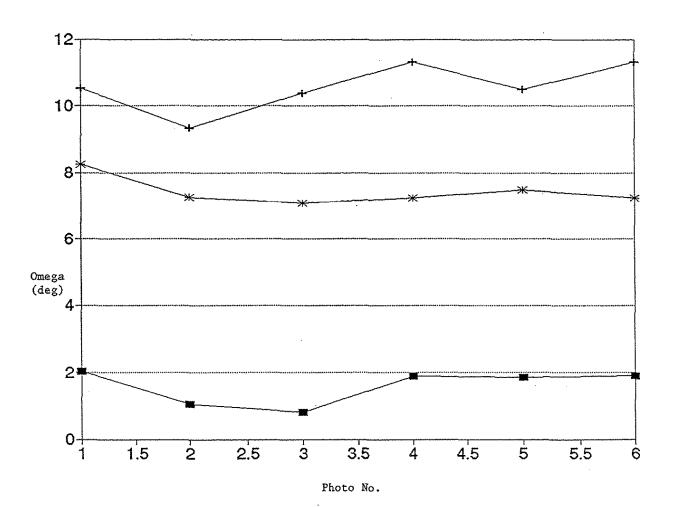
Graph 5.2. Position solution for tail antenna for flight #1.

Table 5.4. Omega, phi, kappa, and scale by GPS.

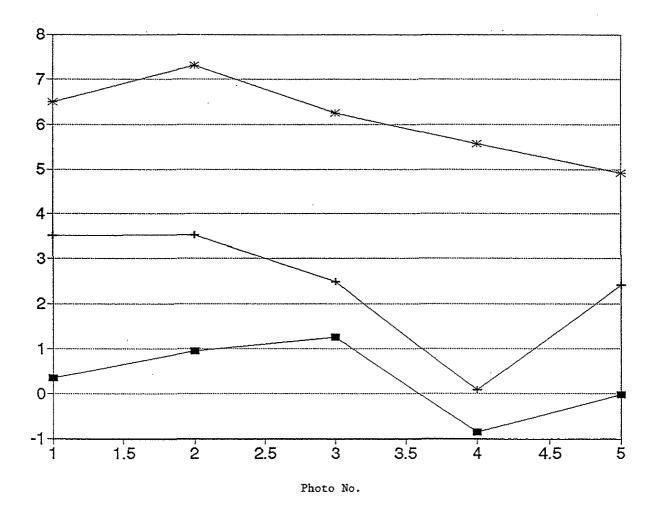
,

TIME	OMEGA	PHI	KAPPA	S1	S2
571817, 572149, 572165, 5726630, 5726630, 5726630, 5726630, 5722635, 5722640, 5722646, 5722646, 5722646, 5722646, 57332009, 5733560, 5733560, 57335570, 57335570, 57335570, 57335570, 57335570, 57335500, 57335500, 57335500, 57336008, 57336008, 57336005, 57336008, 5733600008, 57336008, 57336008,	0.0000, -1.5536, -1.22610, -5.9266, -5.10630, -5.9266, -5.10630, -5.10630, -5.10630, -5.143329, -5.143379, -7.5.14367, -7.5.14367, -7.5.131807, -7.5.131807, -7.5.443329, -7.5.131807, -7.5.441282, -7.5.441282, -7.5.441282, -7.5.441282, -7.5.5016, -7.5.5016, -7.5.5016, -7.5.5016, -7.5.5016, -2.5.5052, -3.5.52849, -2.5.5052, -3.5.52849, -2.5.5586, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5.566, -2.5586, -2.5.566, -2.5586, -2.5.566, -2.5586, -2	0.000, -8.991299, -1.668268, -9.512194, -1.668268, -9.512194, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.668268, -1.66826, -1.66866, -1.66826, -1.6686, -1.6686, -1.6686, -1.66866, -1.66866, -1.66866,	$\begin{array}{c} -81.9887,1\\ -3.5591,1\\ -3.3565,1\\ -3.7456,1\\ -3.3499,1\\ -4.40951,1\\ -5.14695,1\\ -5.12695,1\\ -4.4695,1\\ -5.12695,1\\ -3.2695,1\\ -78.665109,1\\ -78.665109,1\\ -78.665109,1\\ -78.665109,1\\ -77.27326,1\\ -77.27378,1\\ -77.27378,1\\ -77.27378,1\\ -77.27378,1\\ -77.5.7326,1\\ -77.27378,1\\ -77.27378,1\\ -77.27378,1\\ -77.27378,1\\ -77.27378,1\\ -77.2372,1\\ -77.2372,1\\ -77.234,1\\ -77.24,1\\ -77.24$	142206 137844 078808 077550 0777173 0779490 079490 079878 079490 079878 0432455 0432455 0453051 04474601 0474601 0474601 0474601 081692 0836433 1022645 102284 102287 101484 102387 102440 102466 107750 102666 107750 102666 107750 102666 107750 102666 107750 102666 107750 102666 107750 102666 107750 102666 107750 102666 105418	0,1.000000 3,1.0668778 2,1.0668778 2,1.0668778 2,1.0668900 4,0.9398876 5,0.9334898 1,0.9416347 9,0.93890365 3,0.9366953 4,1.3165023 5,1.3172101 1,1.3206856 9,1.3154584 3,1.3215734 5,1.32198155 8,3.7974188 4,3.0052922 3,1.8802952 6,1.4267937 5,1.4176348 4,3.00529222 3,1.8802952 6,1.4267937 5,1.4176348 4,3.00529222 3,1.8802952 6,1.4267937 5,1.4176348 2,1.26878200 5,1.26878200 5,1.26855108 4,1.27788047 6,1.27535200 5,1.2788047 4,1.27722066 5,1.2712036 2,1.2674063 8,1.2688577 5,1.27303505 5,1.27303505 5,1.27303505 5,1.27303505 5,1.27303505 5,1.27303505 5,1.27303505

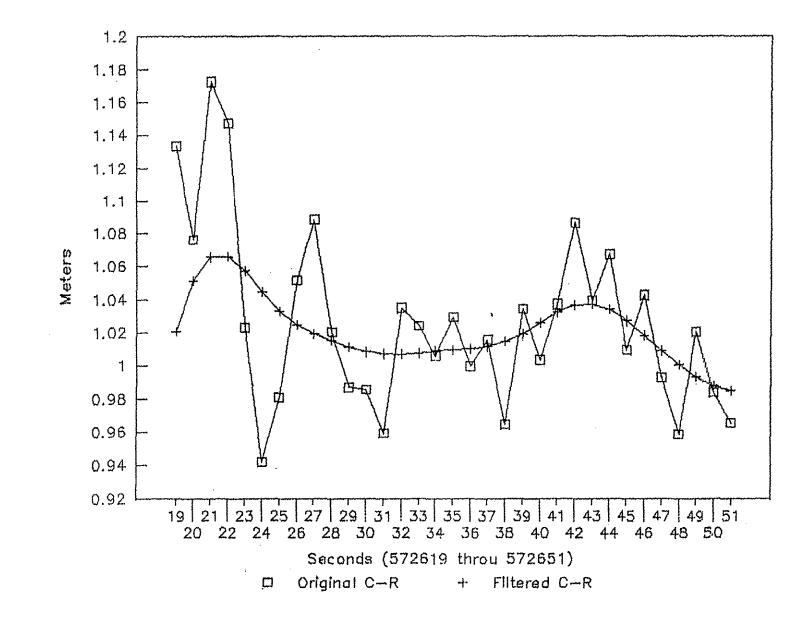
.



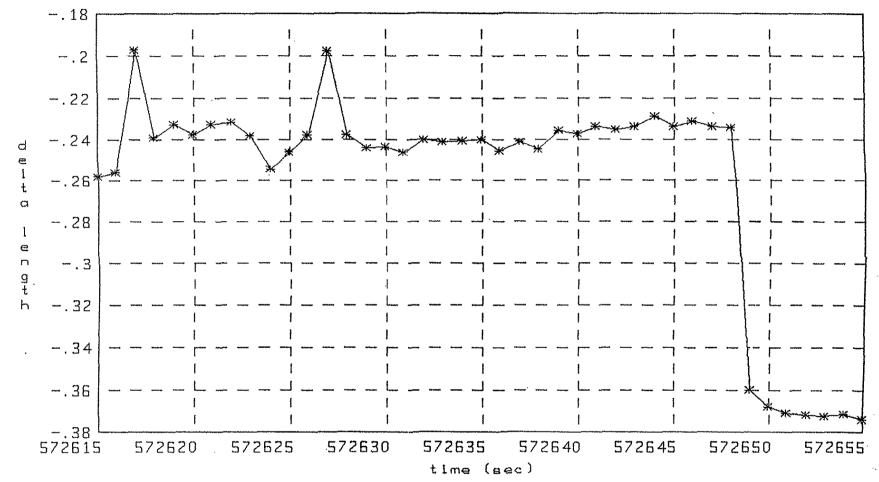
Graph 5.3. Comparison of ω values-high flight



Graph 5.4. Comparison of $\boldsymbol{\omega}$ values—low flight.



Graph 5.5. Original and filtered C-R.



Graph 5.6. Change in length of wing with time.

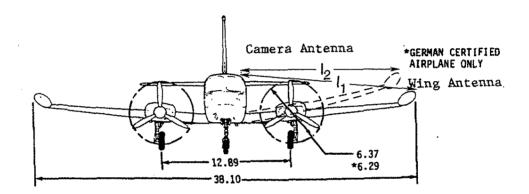
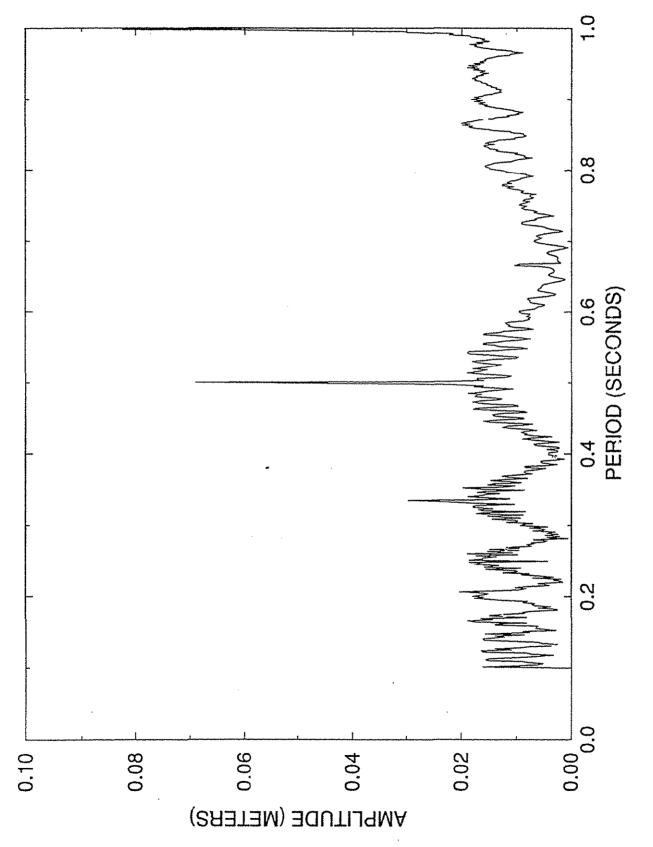
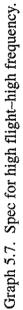
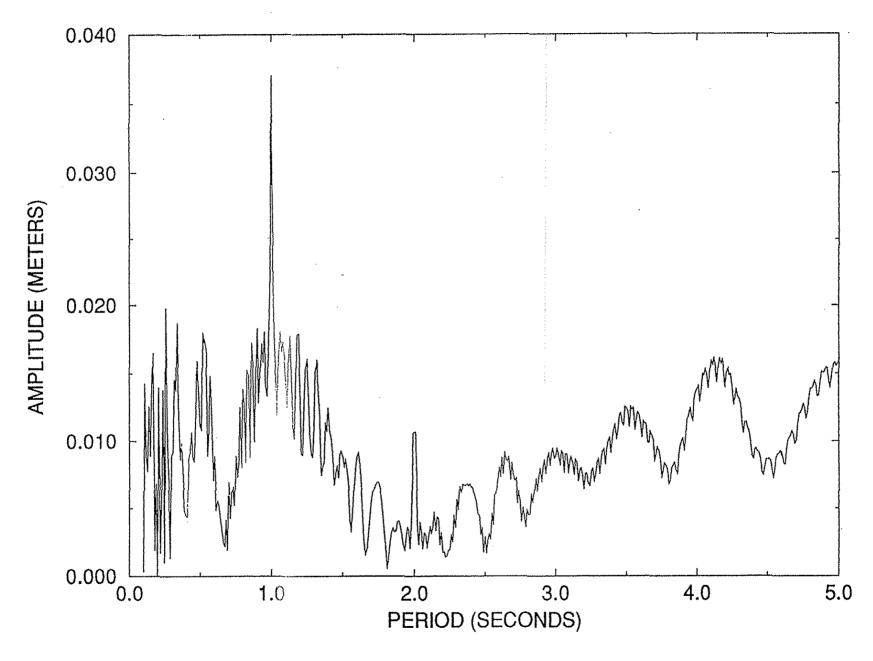


Figure 5.2. Change in scale of wings







Graph 5.8. Spec for high flight-low frequency.

Photo #	OMEGAc(Yao)	OMEGAp(Calib)	OMEGA	Ac (Ken)
		grees)		
			No Deflection	Deflection
41	2.898	1.824	3.085	8.735
42	2.057	0.948	1.110	7.203
43	1.830	0.722	1.562	7.250
44	2.185	1.691	1.768	7.693
45	2.459	1.671	1.736	7.415
46	2.423	1.530	2.147	7.812
. 49	-12.374	0.303	-12.814	-7.095
50	-12.931	0.868	-13.747	-8.070
51	-13.146	1.144	-13.516	-7.834
52	-11.069	-0.778	-11.778	-6.007
53	-11.161	-0.020	-11.283	-5.514
56	-6.087	0.871	-2.784	2.900
57	-7.079	0.244	-4.026	1.659
58	-7.173	-0.002	-4.003	1.679
59	-7.421	-0.270	-4.441	1.236
60	-8.460	-1.295	-5.324	0.342
63	-6.971	-0.191	-3.968	1.592
64	-8.135	0.909	-4.744	0.819
65	-8.326	1.304	-4.703	0.859
66	-8.792	1.378	-5.335	0.225
67	-7.436	0.114	-4.435	1.132

Table 5.5. Comparison of ω values.

*Using GPS time 571382 as initial time.

determine the amplitude and frequency of the flexing of the wing, a spectral analysis was done using the change in length. Graphs 5.7 and 5.8 show the spectral analysis of the high flight. The spectral analysis indicates that it is possible for the wing to vibrate with an amplitude of 10 centimeters and a frequency of 1 Hertz (1 cycle per sec). The amplitude and frequency may vary depending on the speed, flying height, air pressure, wind speed, amount of fuel in the tank, etc. However, it is felt that change in length (Fig. 5.2) will correspond to the extent of deflection and will have an effect on the calculation of the rotation. Thus, the first-order model developed in the first test is modified as

$$w_p = w_o + w_G (A \cos k_G + B \sin k_G) + C D1 + Dt$$

where C and D are constants, Dl is the change in length from the initial set up, and t is the time in seconds from the initial set up.

Table 5.5 shows the omega angle by GPS (both Ken and Yao) and the omega angle by photogrammetric block adjustment using Calib software for both high and low flights. These values are then used to determine the parameters A, B, C, D, and w_o by least squares, giving

 $w_0 = 0.170668$, A = 0.057366, B = 1.305072, C = 0.155838 and D = -0.000074 using Yao's photogrammetric values for both the high and low flights. Using these values, a second-order correction model was developed for the lower flight. Using statistical significance the model was found to be

$$w_{p} - w_{c} = A_{0} + A_{1} X + A_{2} Y + A_{3} t + A_{4} S + A_{5} X^{2} + A_{6} Y^{2}$$

+ A₇ S² + A₈ xt + A₉ xs + A₁₀ Ys + A₁₁ st + A₁₂ t³

where

 $w_c = omega$ computed from the first-order model

 $A_0, \dots, A_{12} = constants$

X, Y = coordinates of the camera location

t = time from the initial set up

 $S = l_2/l_1$, the scale (see Fig. 5.2)

A least-squares fit of this model for the lower flights gave a standard error of 0.00018 radians, indicating a satisfactory solution. This model shows that the first-order model needs to be further corrected for variation in orientation, time, and scale.

Table 5.6 shows the results of a single strip adjustment for the high flight by Calib for two methods. In the first method (which is the present photogrammetric method) only ground controls are constrained; in the second, our proposed method, the nodal point coordinates obtained by GPS and w_c obtained using the first-order model are constrained.

Method	X ₀	Y ₀	Z ₀	Kappa	Phi	Omega	σ ₀	
1			Control)					
	246392.4	307479.9	953.9	0.12388	-0.054975	0.031843	0.15	
Stand. Error	27.4m	33.7m	25.3m	0.0168 (rad)	0.01634 (rad)	0.029 (rad)		
2	(No C	Fround Con	trol, but	Weights or	n X ₀ , T ₀ , Z	₀ & Omega	l)	
	246389.2	307482.1	957.3	0.11825	-0.05574	0.032031	0.5	
Stand. Error	0.6m	0.6m	0.6m	0.0006 (rad)	0.0010 (rad)	0.003 (rad)		

Table 5.6. Comparison of exterior orientation elements.

The difference between w by the two methods is 0.00019 radians, which compares well with the value obtained by the second-order model, indicating that the relative orientation and scaling between photos in the Calib software automatically compensates for the second-order correction.

Table 5.6 also shows that the standard error of the exterior orientation elements of the second method is better than that of the first method. So the first method is better than the second because the weight on w for the second method is 10,000 (this is required to constrain w within ± 0.0001 radians). However, the maximum photo coordinate residual (25 microns) for the second method is better than the first method (27 microns), indicating the ground coordinates are better for the second than the first.

6. ANALYSIS OF FINAL TEST

6.1. ISU and Highway 30 Project

The final project to test the concept in the airborne GPS was done from June 1994 through February 1995. Previous studies have shown that reliable GPS data can be collected with antennas at the fuselage and wing locations of the aircraft. For the fast static data processing, we need at least seven satellites and L_1/L_2 receivers. For reliable photogrammetry observations, we need targeted control and pass points. Thus, for the final project, the aircraft was fitted with four antennas; two on the wings, one on the fuselage, and one above the camera (see Fig. 2.7). Six state-of-the-art GPS Z12 receivers were used; four in the aircraft and two on the ground base stations.

Two test photographic sites were selected: one on the ISU campus and the other between Nevada and Colo on Highway 30.

6.2. Ground and Flight Control

The ISU test site consisted of nine pre-targeted points (see Fig. 6.1). The coordinates of the targeted points were determined by GPS with an Ashtech L_{12} GPS receiver, using Iowa DOT as the reference point. The Geolab software was used to adjust the vectors.

The Highway 30 test site consisted of a number of pre-targeted points (see Fig. 6.2). The four control points (NW, NE, SW, SE) were established by Ashtech L_{12} GPS receivers using the Town Engineering Building and the Iowa DOT as reference points. The Geolab software was used to adjust the vectors (see Fig. 6.3). The control points along Highway 30 were established by the use of a total station traverse using the four corners (NW, NE, SW, SE) as control points. The points along Highway 30 were painted and the pass points in the field were targeted prior to flight.

The Base 1, Base 2, and Taxi, used for flight control at the Ames airport, reference points were established by Ashtech L_{12} GPS receivers using Town Engineering Building and the Iowa DOT as reference points. Geolab was again used to adjust the vectors (see Fig. 6.4).

6.3. Flight Mission

On June 20, 1994, the Cessna aircraft fitted with four L_1/L_2 antennas and a L_1 antenna for navigation was used to test the airborne GPS concepts (see Fig. 2.7)

The aircraft was taxied over the Taxi point; the four GPS Z12 receivers were connected to the L_1/L_2 antennas and arranged to collect the data on flight (see Fig. 6.5). Two Z12 GPS receivers were set on the nearby reference points Base 1 and Base 2 (see Fig. 6.6).

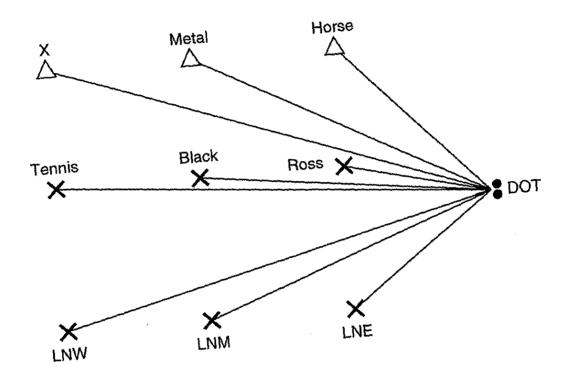


Figure 6.1. ISU campus control.

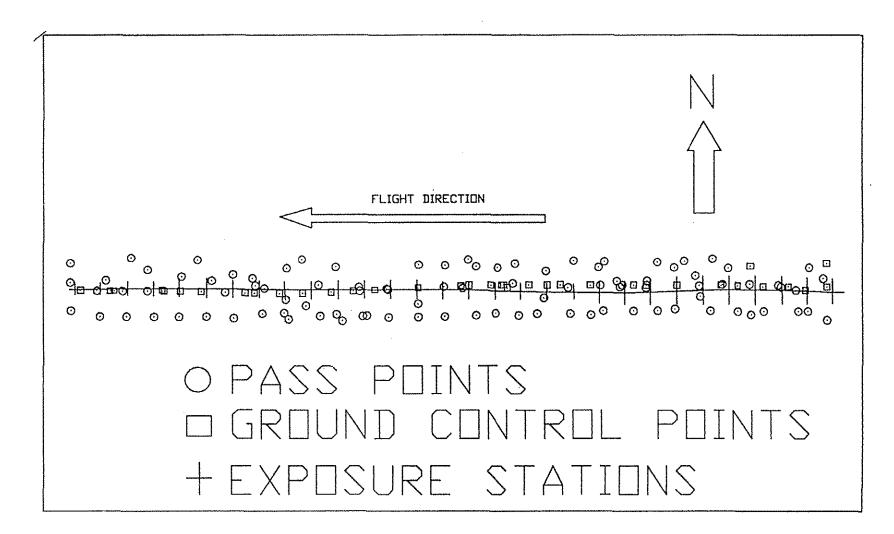
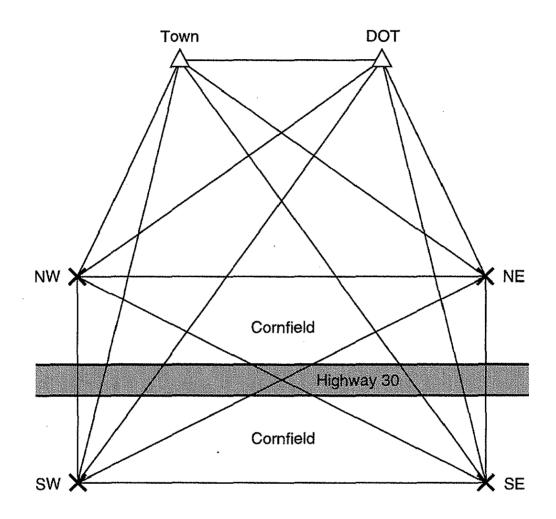
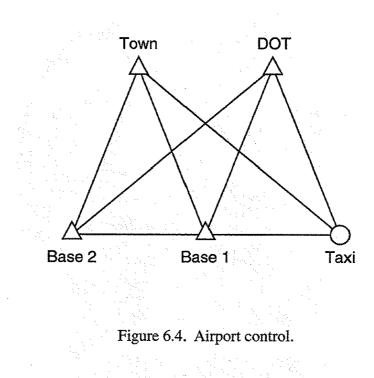


Figure 6.2. Highway 30 flight, 1994.



i.

Figure 6.3. Highway 30 control.



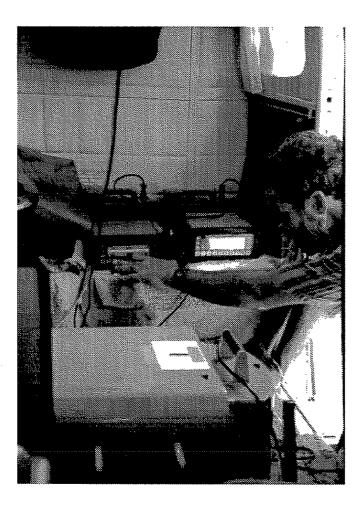


Figure 6.5. GPS receiver arrangement in the aircraft.

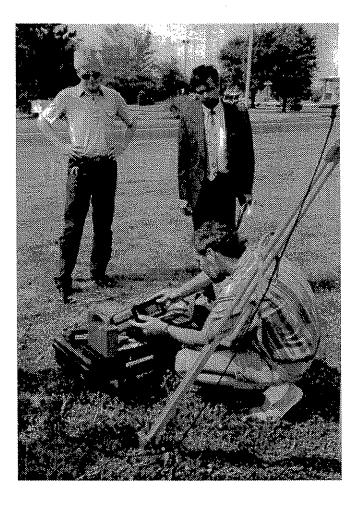


Figure 6.6. Base station GPS data collection

The flight plan consists of one flight in the east - west direction at a flying height of 3,000 feet over the ISU campus, and another over the ISU campus and continuing over the Highway 30 test site at a flying height of 1,500 feet (see Fig. 6.7). The campus site is 3 to 5 kilometers from the airport and the Highway 30 site is about 17 to 30 kilometers from the airport.

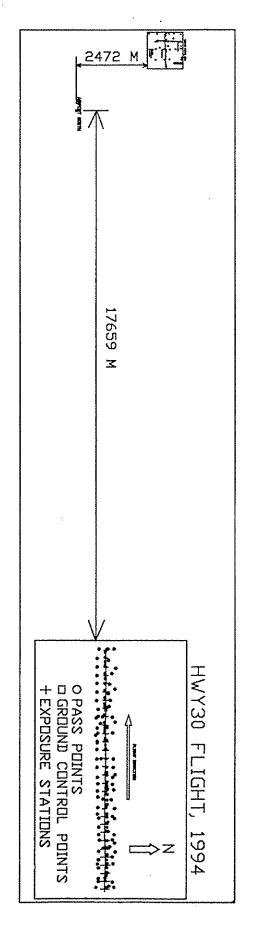
Each flight collected data for a few minutes at the Taxi point, then flew over the test site, took photographs at the pre-determined location using pinpoint navigation (see Figs. 6.2 and 6.8), then returned to the Taxi point, and collected additional data for a few minutes. During the entire mission, GPS data were collected every half second by the four receivers on board and the two receivers on the ground base station (see Fig. 6.9). The LMK 2000 camera was used on this flight mission, and the exposure times were also recorded on the GPS receiver.

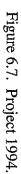
6.4. Processing of GPS Data

The GPS data were processed using Prism (a new version of GPPS) and PNAV software developed by Ashtech. The data from the receivers were downloaded to a personal computer network and the position of the aircraft antennas with respect to both base stations were determined. Also, the position of the left and right antennas with respect to the camera station was processed. The data were processed both in the forward and reverse direction, which allowed the software to eliminate any integer ambiguity.

The results were smooth and the positions of the antennas with respect to all three references agreed within acceptable limits. Figure 6.10 shows the location of the left wing, right wing, and camera antennas with respect to the Taxi point. The difference between the camera antenna coordinates determined by PNAV when the aircraft is over the Taxi point and the coordinates from the control survey is 0.06 meters in x and 0.13 meters in y, indicating that the PNAV position determination is accurate. The small difference shows the pilot's ability to taxi the aircraft exactly over the Taxi point. The height of the camera antenna above the camera's nodal point given by PNAV and the tape measurement is 1.541 meters, which compares with the previous calibrated value of 1.464 meters. The difference is due to the use of a cloth tape for measurement and the lack of knowledge about the exact location of the nodal point.

The PNAV software gives the antenna location in spherical coordinates (ϕ , λ ,h) or in local three-dimensional coordinates (X,Y,Z) with respect to the reference points. PNAV also interpolates the position of the antenna at the exposure time of the camera. For practical applications, the spherical coordinates can be converted to the state plane coordinate system. The angle omega (assuming the y axis is parallel to the camera and to the left and right wing antennas), the angle phi (assuming the x axis is parallel to the camera and the front antenna), and the angle kappa (assuming the z axis is parallel to the vertical) can be calculated (see Fig. 6.11). Table 6.1





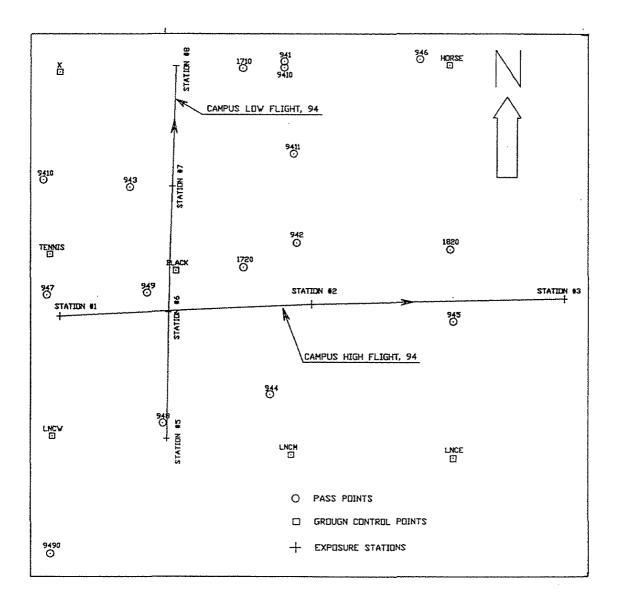
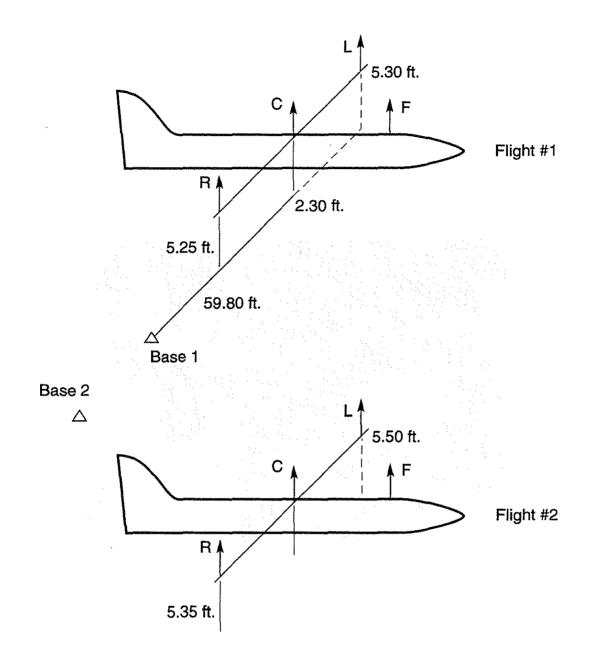
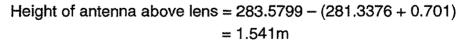


Figure 6.8. Campus flight 1994.

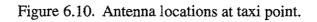


Figure 6.9. GPS data collection on photo mission.





	Х	Y	Scale		Con	vergence	
BS2 2, AIRP 3,	1055444.7742,	1489846.9346, 1489831.4460, 1489850.9376,	1.0000129530,	 0,	4, 59.4467		1401 1401 1401
PNAV } AIRP3 }	1055358.2885,	1489850.8052, 0.1324					



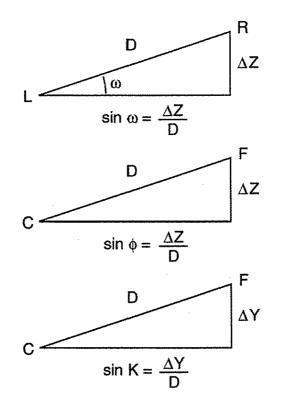


Figure 6.11. Computation of ϕ_G , ω_G , k_G .

									PREFLICT to									
2 4 4 4	F-C & L		# 60 C	₽ 60 C Distance	R EG L	CHARGEAT	GHOOL	CHECKLEY.	4WE	erbart.	St.Ppt.r	KAPPAL	5Å9931x	SCALE3-C	SCALL/C	BCALLER	SCALET 1	TIME
1 Diff 1 0166	1 20101 1 21					+5.2067114	7 4911514	1 1717743	2 4134493	1 1165761	L 3071841				0.99901196	1	0.0003608	348196.00
-9.475 -0.784		00 5.4331430						3.5005851	2.4134359	6.9903775	1.3431731	1.4438353	7.4239254	0.00041147	0. 99742055	1.00000344	0.99144373	148194.30
+0.423 -0.749	0.043 -0.2	51 5.6523993	5,4215854	9.99434416	11,203141	-5.0240257	7.4544535	1.2240313	3.6179534	4.3361306	2. 8726684	4.9612614	7.1471785	0.99947645	0. ******	2.0004 17	0.99960539	245193.00
+0.681 -0.776	0.068 -0.3	95 5.4353803	5.0244999	0.99764823	11,306364	-4.8790183	7.9302443	1.5061752	3.9074646	5.9341.55	7.5046032	6.5399954	4,7839578	1	1	1	\$	248197.50
-0.442 -0.821	0.643 .0.	37 5.6548275	5.4352444	0.97701037	11.209206	-4.473637		2.892594	7.62357#3	5.0544455	7.233143	4.3338172	4,6342575	0.99990583	1.0001384		1.0000734	148196.00
-0.47 +0.834	0.061 -0.4	04 5.6573334	5.4541241	0.99643455	11.10101	-1.337394	0.5247897	1.0451757	2.5020364	5,00016	7.1347244	4.1309002	4.4427227	3.0003109	3.0002848	1.0007578	1.0003451	348198.30
-0.418 -0.873	0.035 -0.3	18 5.6255835	5,6761265	0.19754477	11 207 95	-4.1072121	8.804804	1.6334703	1 5614674	5.1414916	4. 14110/0	5.7590662	3,89400[4	1.0000391	0.999993539	0.99946554	0,9999474 1.0000549	148199.00
-0.332 -0.866	0.054 -0.4	75 5,6553075	5 6164083	0.9970331	11 210254	1. 1445712	1. 8519924	1.4744593	1.1141101	5.5768725	6. 21295.41	1.1316225	5 31 97 997	0.999799000	1,0003411		1.0001467	140200.00
-0.401 -0.961	0.054 -0.	46 5.4347 203	5.4361297	0.99712266	11.310954	~4.D450074	8.8039051	3.3515798	1.1014314	5.4506.175	7.0743439	4.1014139	4.3511094		1.0003915			148200.50
-0.38 -4.573	0.057 -0.4	33 5.6572602										5-4042233			4.99864347			244202.00
-0.341 -0.517		34 3.4584434										4. 4109387			0.99916217			148201.50
-0.252 -0.202		\$5 \$.\$\$75602 \$1 \$.6576478						2.227977	7.2554527	2.0000000	3.4743097	4.20063	4.5035157		0. ###\$1575			149202-00
-0.365 -0.879		31 9.6329319												2.0004045	0.99972432	0.3000001	0.99990347	148202.50
-0.423 -0.517		10 3.4549492							3.4543516	3.94443	3.2071433	4.4344244	4.6631374	0.999933322	0.9193893	0.99792351		144203.30
-0.402 -0.575	0.041 -0.4	36 3.654873	5.4218012	0. 196 (74)	21.307173	-4.0745471	8.5724076	1.2295784	3.1093884	3.660141	5.0123077	4-1414942	4.3753714	0.99991351	0. 999522		0. 22249174	348204.00
-0.38 -0.64	0.041 -0,	48 3.654068	\$, 4203522	0.99423423	11,2044,77	~3.8533754	\$,4017113	2.4547444							0. 79724434		Q. 99 044192	348204.30
-9.372 -0.667		94 5.4552 8 44				-3.7418301									0.99946673		D. 3998806T	144205.00
-0.397 -0.645		48 3.4550401	5.4119397			-4.0254167						3.4776099			0. ###37133		0.97980446	148203-50
-9.362 -0.882	0.043 -0.4		3.4122107			-3.2037395						2.3333397			0.999941294	2.0010054	0.39771413	148206.00
-0.303 -0.927		25 3.4534466				-1.0619951									0	1.0031845		348207.90
-0.338 .0.904		44 5.4547631												0.79949435		1.001007	1.000013	348307.30
-0.367 -0.869	0.06 -0,5	00 5.6527404	5.6339717			-3.3325045								0.99953477		1.0030919	0.99976776	148206.00
-0.409 -0.834		27 3.4540893				-4.3683373									0. 37 974213		0.99968345	148308.50
-0.403 -0.839		27 3.4527768				-2,0773373									6. \$9942777		3. 28976495	£68309.00
-0.407 -0.435		28 9.4543471 28 9.4543437				-4.1378524									1.0000711		1.0000447	348309.30
-0.412 -0.136		04 5.4539944				-4.3804001								C. #9975737	1.000048		1.0000186	148110.00
-0.631 -0.43		99 5.4523285				4,1723503									2,0001417		0. 21753475	368311-00
-0.404 -0.833	0.054 -0.4	24 5.4573045	3.4320574	0.99923472	11,204938	-4.339398	8.5103768	3,1748526	J. 2116334	1.7073744	3.063033	2.1330054	3.2989064	0.99943433	0.39954754		0. 99949343	14\$711.50
-9.437 -0.155		68 S.45339644				-4.3327445									0. 99 874 844		0.99973313	148333.00
-0.463 -0.775		16 5.6541763				-4.6868338								0.99979102		7.0033838		14\$332.50
-0.491 -0.746		55 5.4352074 67 5.6546266				-4.9807512								0.99999068	0.99963155	1.003178	1.000347	144313.00
-0.511 -0.72		0 5.4552393				-5.1847415								0.99997843		1.0025435	1.0001007	148314.00
-0.678 -0.443	0.067 0.0		5. 594544					-0.076432574								1.0066009	1.0037438	149750.00
-0.477 -0.444	0.06 0.0	13 5.4342774						-0.0662371275							1.0121443	1-0034039	1.0032716	148350.50
-0.643 -0.656		26 5.6336925						-0.13349244								1,004743	1.0031417	148751.00
-0.658 -0.672		19 5,4345297						0.096124721								2-0042294	1.0631111	248792.30
-0.641 -0.675		14 5.4344063						0.071343014							1.0111496	3.0058109	1.0038463	148752.00
-0.454 -0.449		11 3.4304397			11.340143			0.056067998								3-0040414	1.0036994	140352.50
-0.674 -0.647	9.054 0.0		3,4471349					-0.13743328								1-0063007	3.0029934	140152.50
-0.669 -0.663	0.034 0.0		5.4941447	1.0013354				-0.030573373	2.0912623	-45 473454	47.777871	44.400185	47.598249	0.00573597	1.0113917	1.0024948	1.003305	140754-00
-1.456 -0.449		49 5.7263356				-14. 600162						44.563359		3.036335	1.0115503		0.99393998	148754.50
-0.453 -0.727		74 5.6389184				-6.6617734								0.99532449			0,9973544	140755.00
-0.457 -0.733		75 3,625000				-6.6954959								0.99640039			0.99705066	144755.50
-0.474 -0.73		44 3.4366781 38 5.6358107				-6.8699454								0,99624393	1.0017312		0.99734429	148756.00
+0.459 -0.134		77 5.4335144				-6.7200524								0.9957941		1-0061649		148357.00
-0.452 -0.143		91 5.4320725				-6.647783									0.99992073		G. 99418553	148257.50
-0.449 -0.739	0.045 -0.	09 5.6396048	5.4144394	1.0053128		-4.4199716		0.46213352	3.545514	-48.142994	-50.880834		~\$0,0321	0.99544585	0. ********	1.0074705	0.99954367	148758.00
-0.661 -0.129		64 5-6302784				-4.7635259									0.99842245		8. 99424492	248752.30
-0.63 -0.766	0.04 -0.1	36 5.4327958	2.0091413	1,0063225	13.145301	~+.4238373	7.4540373	0.09910135	2,276674	-77.779755		-10.729084	41.716968	0,79583328	0.#943433	1.0050737	0.99437839	148759.00

Table 6.1. Results of flight 1 for entire time.

-

shows the time, antenna locations, and angles at all times of flight prepared by using a spreadsheet. Table 6.2 shows the results for the camera exposure times.

For this study, it was sufficient to accept the data with Base 2 as a reference and the interpolated antenna positions given by the PNAV software.

6.5. Photo Coordinate Observation

The photo coordinates of flight 1 and flight 2 were observed using a Wild STK1 stereo comparator. On the campus site, only controls were targeted and the natural points were used as pass points. Wherever possible, the same natural points were used for flights 1 and 2.

On the Highway 30 site, most of the painted targets along Highway 30 were clearly seen but some points on the side roads were obstructed by construction activities. Most of the targeted pass points on the south side were clearly seen but most of the targeted points on the north side were not, perhaps as a result of overgrowth of the corn and shade caused by the sun.

The coordinates were observed by two graduate students. The comparator coordinates were processed by Sat9 software to get the photo coordinates. These photo coordinates were then processed by RO software for agreement between adjoining photos. Finally, the coordinates were processed through Albany software for agreement between strip and ground coordinates. At each stage, if there was disagreement, the coordinates were re-observed and the inconsistencies eliminated. Again, in this study, errors due to refraction and lens distortion were assumed to be negligible.

6.6. Analysis of the Flight Data

The output of the Albany software, namely, the approximate ground coordinates, the camera location and orientation, and the photo coordinates, were used as the input into the Calib software. The Calib software is a self-calibration software that runs on Project Vincent, a state-of-the-art computer network with Unix workstations.

Table 6.3 shows the comparison between results by Calib (photogrammetry) and PNAV (GPS). In this table, Photos 1–3 are from flight 1; Photos 4-7 are from flight 2 (campus site); and Photos 8 and 9 are from flight 2 (Highway 30 site).

Unfortunately, many of the targeted pass points north of Highway 30 and some of the targeted controls on the side roads were not visible on the photo. Due to this, the accuracy of the Highway 30 site strip was questionable. Thus, the results of only the first two photos on the Highway 30 site were used in the analysis.

Table 6.3 also shows that the difference in Z is large for the campus site. Upon investigation it was found that a focal length of 152.44 was used instead of 152.21, that the camera

Table 6.2. Data of flight 1 for exposure time.

TIME		CAMERA			LEFT			RIGHT			FORE	
	Xc	Ye	Zc	X1	Y1	Z]	Xr	Υr	Zr	Xf	YÍ	Zf
148201.670116												
148207.775577												
148213.882438												
148754.715104	-39.466	-1380.126	9.324	~34.459	-1382.474	8.098	-44.608	-1377.851	8.630	-39.908	-1381.025	9.376

	X	Y	Z	Dist. from Ref. Pt.	Flying Ht.	Direction	Geoid Undulation
1 2 3	-0.242 0.385 0.709	-0.367 -0.606 -0.344	$ \begin{array}{c} 4.102 \\ 4.254 \\ 4.295 \end{array} $	3.5 km	3,000 ft f=152.442	W -> E	28.87
4 5 6 7	0.265 0.361 0.317 0.1962	0.502 0.262 0.042 -0.124	$ \begin{array}{c} 4.0632 \\ 3.678 \\ 3.418 \\ 3.365 \end{array} $	3.5 km	1,500 ft f=152.442	S –>N	28.87
8 9	-1.050 -1.25	-0.984 -0.862	1.473 } 1.025 }	26 km	1,500 ft f=152.212	E> W	29.45

Table 6.3. Photo-GPS location.

location by GPS was not corrected for antenna height, and that the elevations of control points, because they were determined using L_{12} GPS receivers, were not accurate.

In addition, Table 6.3 shows that the difference in x and y is large for the campus site. Upon investigation it was found that the ground control for Highway 30 was on a surface state plane while the GPS was on a state plane. The side road controls at the end of the strip were covered by construction. Corn obstructed the targeted and pass points north of Highway 30.

Table 6.4 shows that the difference in orientation angles between the photogrammetry and GPS methods was consistent for the campus site in flight 1 and flight 2 but not consistent between the campus site and the Highway 30 site in flight 2 for the following two reasons:

- (1) The Highway 30 site is about 25 kilometers from Base 2, which is beyond the acceptable limit of 10 kilometers for reliable ambiguity resolution by the PNAV software. The campus site is only 5 kilometers from Base 2 and the ambiguity resolution is reliable.
- (2) Lack of good targeted pass points and control points at the end of the Highway 30 strip.

As discussed earlier, to do strip adjustment using airborne GPS without any ground control, the omega orientation angle needs to be determined by airborne GPS. Previous tests have shown that:

$$\omega_p = \omega_o + (a \cos K_G + b \sin K_G)\omega_G + c\phi_G$$

where

 ω_p = omega by photogrammetry

 $\omega_{\rm G}$ = omega by GPS

 $\omega_0 = a \text{ constant parameter}$

Table 6.5 shows the results of the least-squares fit of the above equation using the campus site flight 1 and flight 2 data. The standard error of 0.0005 indicates that the accuracy of ω_0 is better than or equal to 0.0005 radians and is acceptable for highway application using 1,500 feet or 500 meters in flying height photos. The following reasons support the conclusion:

(1) As discussed earlier, the accuracy of phase measurement in the Z12 receiver is 0.2 millimeters. Assuming a noise of 0.2 millimeters due to multipath etc., the relative error in the z direction between the left and right wing antennas can be assumed to be about 0.5 millimeters. Because the distance between the left and right wing antennas is about 10 meters, the error in ω_0 is = 0.00005 radians.

Table 6.4. GPS-Photo orientation.

ω	φ	к
0.0045	0.0151	0.0032
0.0053	0.0130	0.0024
0.0037	0.0117	0.0019
0.0045	0.0133	0.0025
0.0006	0.0014	0.0005
0.0055	0.0168	0.0076
0.0071	0.0255	-0.0117
0.0054	0.0177	-0.0010
0.0036	0.0188	+0.0089
0.0055	0.0197	0.00094
0.0012	0.0034	0.0082
0.0116	0.0027	-0.0061
0.0118	0.0029	-0.0057
0.0117	0.0028	-0.0059
0.0001	0.0001	0.0003
	0.0045 0.0053 0.0037 0.0045 0.0006 0.0055 0.0071 0.0054 0.0055 0.0012 0.0116 0.0118 0.0117	0.00450.01510.00530.01300.00370.01170.00450.01330.00060.00140.00550.01680.00710.02550.00540.01770.00360.01880.00550.01970.00120.00340.01160.00270.01180.00290.01170.0028

Table 6.5. Results of combination of high and low flights.

INPUT DATA ARE: (OMEGAP, OMEGAg, KAPPAg, PHIg)

.

OMEGAp	OMEGAg	KAPPAg	PHIG	
0.043108000 0.042177000 0.015774000 0.036485000 0.047588000 0.011352000 0.007231000	0.047640007 0.047439621 0.019469214 0.042290545 0.054703783 0.016753616 0.010923818	0.075824965 0.044556999 0.041470049 0.045362608 0.023387758 0.052766161 0.067033636	-0.061304218 -0.060995646 -0.057373606 -0.030498599 -0.023387548 -0.040786984 -0.033132291	
FORMULA IS OMEG	Ap-OMEGAg=OMEGAo	+OMEGAg (a*COS (KAPPAg)+b*SIN(KAPPAg))+d*PHIg
-0.0048850666	-0.0641132373	0.4197135928	-0.0300743669	
THESE ARE THE	ERRORS, (COMPUTE	D - REAL)		
-0.000040 0.001041	0.000060 -0.000587	-0.000373	-0.000066	-0.000035
THE STANDARD D 0.000513	EVIATION IS			

- (2) The elevation of the control points in the campus site are determined by the L_{12} GPS receiver from the Iowa DOT. Thus, the relative error of the ground control is about 0.1 meters, and therefore, the error in Wp due to ground control for the lower flight is about = 0.0001 radians.
- (3) The error in elevation by photogrammetry due to error in the photo coordinate, Δr , is given by:

dH = (H/r) * dr

Thus, if r = 100 millimeters, dr = 0.01 millimeters, and H = 1000 meters for flight 1, then dH = 0.1 meters.

The error of 0.1 meters in dH will result in an additional error of 0.0001 radians in ω_p due to photogrammetry and this together with the ground control error will result in a total error of 0.0002 radians in ω_p (see Fig. 6.12).

(4) An error of 0.0005 radians in ω_G for an airborne GPS without any ground control will result in an elevation error of 0.25 meters for flight 2 (500-meter flying height). Thus the elevation error is good for drawing one meter contour at either flying height.

Table 6.5 shows the transformation parameters for transferring ω_a to ω_p obtained from campus site data. They are not suitable for the Highway 30 site perhaps for the following reasons:

- 1. The Highway 30 site is more than 10 kilometers from the reference base station, which is beyond the acceptable limit for integer resolution by PNAV.
- 2. The difference in the geoid undulation at Base station and the Highway 30 site is about 0.7 meters and that between the base station and campus site is 0.1 meters, suggesting that there may be a large difference in the deviation of the vertical between the campus and Highway 30 sites.
- 3. The direction of flight 1 is from west to east and the direction of flight 2 over campus is from south to north, while the flight over Highway 30 is from east to west. The different directions and time of flight may result in a different error in W_G due to different multipath errors in the left and right wing antennas. The differences between W_G and W_p for flight 1 and flight 2 over the campus site agree, suggesting that the multipath error is negligible and the asymmetrical motion of the left and right wing is also negligible. Also, the shape of the aircraft at the location of the antenna is a crest; therefore, any reflection will be away from the antenna, resulting in negligible multipath.

To test the feasibility of using the transformation parameters from the campus site to the Highway 30 site, a combined adjustment of flights 1 and 2 was done using the Calib software. By trial and error, a satisfactory solution was found by assigning different weights to interior orientation elements (x_0 , y_0 , f) (see Table 6.6), to airborne GPS coordinates (X_c , Y_c , Z_c) and to

75

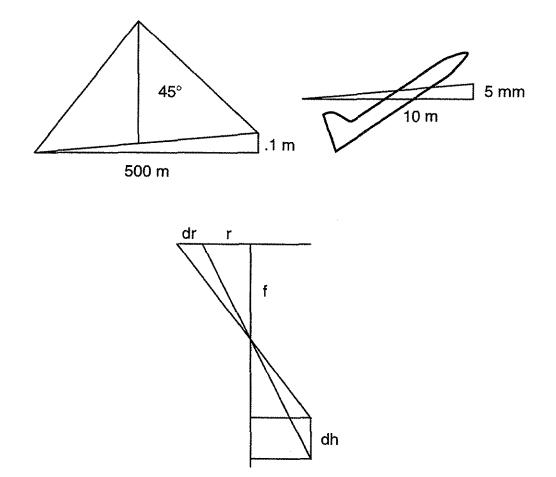


Figure 6.12. Sources of error in ω_P

Table 6.6. ω_G to w_p using different weights.

Weight on photo coordinates = 5000

Standard error on ground contact = 0.01m

Standard error on Airborne GPS (low flight)

$$X_{G}, Y_{G} = 0.01m$$

 $Z_{G} = 0.001m$

Standard error on Airborne GPS (high flight) $X_G, Y_G, Z_G = 0.01m$

FORMULA IS OMEGAp=OMEGAo+OMEGAg(a*COS(KAPPAg)+b*SIN(KAPPAg))+d*phi

-0.0107795884 0.8991249682 0.7631507940 -0.0508904326

THESE ARE THE OMEGAP:

-0.0128428	-0.0205923	-0.0156900	-0.0372186	-0.0150666	-0.0230069
-0.0091735	-0.0128985	0.0087594	-0.0011218	-0.0134105	-0.0244845
-0.0076987	-0.0044199	-0.0056232	-0.0213606	-0.0143648	-0.0108987
0.0026834	-0.0073671	-0.0023437	-0.0119827	-0.0178356	-0.0114358
-0.0118249	-0.0080444	-0.0273579	-0.0311399	-0.0261177	-0.0109928

ground control. The parameters from the campus site were used to obtain ω_p from ω_G in the Highway 30 site strip. When these values were used in the Highway 30 site strip adjustment, even without ground control, they gave satisfactory pass point coordinates, suggesting a self calibration for a site (e.g., Highway 30) can be used to convert ω_G to ω_p .

Table 6.4 shows the error of $(K_p - K_G)$ is about 0.0005 radians and $(\phi_p - \phi_G)$ is about 0.001 radians even though the distance between the camera antenna and the forward antenna is only 1 meter, suggesting that the relative error of GPS coordinates is better than 1 millimeter and that ϕ_G and K_G can be used to rectify aerial photos and also produce orthophoto. The error in $(K_p - K_G)$ is better than $(\phi_p - \phi_G)$ because the determination of K_p by photogrammetry is more accurate than ϕ_p .

6.7. Analysis of Refined Test Flight Data

Because of the possibility of small errors in the initial data, the following steps were taken to refine the ground control, the photogrammetric coordinates, and the GPS data.

- 1. Two ground control points at both the ISU Campus site and the Highway 30 site were connected to the Base 2 reference point using L_1 and L_2 GPS receivers. This procedure eliminated any possible constant, rotation, and scale errors between GPS and ground control.
- A spirit leveling was done between ground control points in the campus site and between the three control points of the first model of Highway 30. This process eliminated any relative error greater than 5 centimeters in elevation between the ground control points.
- 3. The photo coordinates of all the points in the nine photos were re-observed by three observers. Each did observations twice and the mean of the six observations was adopted. For obscure reasons there was a constant error of 0.040 millimeters between the initial coordinates and the refined coordinates at a few of the observed points.
- 4. The GPS coordinates of the antennas were corrected for antenna heights and geoid undulation.

The refined data for the nine photos were then adjusted by Calib. The difference in camera coordinates for the campus site (Photos 1–7) (see Table 6.7), clearly show that the airborne GPS coordinates are better than 10 centimeters irrespective of the flight altitude and flights. The error in the z direction of 0.8 meters for the Highway 30 site is probably due to integer ambiguity resolution by the PNAV software because the Highway 30 site is more than 10 kilometers from the reference station Base 2.

	Х	Y	Z
1	-0.081	0.277	0.008
2	0.305	0.205	-0.148
3	0.005	0.491	-0.167
Mean	0.076	0.324	-0.102
Std. Error	0.165	0.120	0.078
4	0.048	0.343	0.258
5	-0.004	0.1	0.081
6	-0.099	-0.204	0.046
7	0.059	-0.359	0.131
Mean	0.001	-0.03	0.124
Std. Error	0.062	0.27	0.080
8	-0.243	0.286	-0.916
9	0.467	0.249	-0.865
Mean	0.112	0.267	-0.891
Std. Error	0.335	0.018	0.025

 Table 6.7. GPS-Photo locations using refined data.

ţ

Table 6.4 shows that the difference in orientation angles between GPS and photogrammetry is constant for flight 1 and flight 2 on the campus site. However, the orientation angles from GPS for the Highway 30 and for the campus site appear to be different. Again, this is because the Highway 30 site is more than 10 kilometers away from the reference station Base 2, suggesting the importance of having the reference station within 10 kilometers of the site or of knowing the elevation difference for two or more points in the y direction perpendicular to the flight to determine the transformation parameter when obtaining ω_p from ω_G .

Table 6.8 shows that the standard error of the fit between ω_p , from refined data and ω_G is 0.00008 radians. The accuracy of 0.0001 radians in ω is sufficient for drawing 2-foot contours either from 1,500 or 3,000 feet flying height photos.

Table 6.9 shows the difference between $\Delta \omega_1 = \omega_G - \omega_p$ of flight 1 and $\Delta \omega_2 = \omega_G - \omega_p$ of flight 2. The table also shows the second difference, $\Delta \omega_{12} = \Delta \omega_1 - \Delta \omega_2$. The standard error of $\Delta \omega_{12}$, is 0.00003 radians, which agrees with the expected error of 0.00002 for a height difference of 0.2 millimeters at 10 meters apart.

Table 6.8. ω_G to ω_p using refined data.

INPUT DATA ARE: (OMEGAP, OMEGAG, KAPPAG, PHIG, TIME)

OMEGAp	OMEGAg	KAPPAg	PHIg	SECONDS
0.042392	$\begin{array}{c} 0.047640\\ 0.047440\\ 0.019469\\ 0.042291\\ 0.054704\\ 0.016754\\ 0.010924 \end{array}$	0.075825	-0.061304	0.670116
0.041186		0.044557	-0.060996	6.775577
0.014783		0.041470	-0.057374	12.882438
0.036995		0.045363	-0.030499	1457.599484
0.048136		0.023388	-0.023388	1460.871496
0.012002		0.052766	-0.040787	1464.134522
0.007390		0.067034	-0.033132	1467.263800

FORMULA: OMEGAp-OMEGAg=OMEGAo+OMEGAg(a*COS(KAPPAg)+b*SIN(KAPPAg))+d*phi+ E*Kp+F*T

0.0000550947 -0.0763984723 0.5237227962 0.0729936964 0.0129816652 -0.0000011097

THESE ARE THE ERRORS, (COMPUTED - REAL)

0.0000727 -0.0000867 0.0000140 -0.0001274 0.0000901 0.0000744 -0.0000370

THE STANDARD ERROR IS 0.0000859

 \dot{t}

Flight	Photo	Omega GPS-Photo (radians)	Average	Difference 1st & 2nd	Average
1	1 2 3	0.005242 0.006254 0.004686	0.005396		
2	4 5 6 7	0.005296 0.006568 0.004752 0.003535	0.0050375	0.000048 0.000314 0.000066	0.000142

Table 6.9. First and second difference in ω_p - ω_G

7. APPLICATIONS OF AIRBORNE GPS

Airborne GPS has three main applications in photogrammetry: (1) rectifying aerial photos, (2) producing orthophotos from aerial photos, and (3) stereo plotting without ground control.

7.1. Rectifying of Aerial Photos

An aerial photo can be rectified using the equations,

$$x = f * \left\{ a_{11}(X - X_0) + a_{12}(Y - Y_0) + a_{13}(Z - Z_0) \right\} / \left\{ a_{31}(X - X_0) + a_{32}(Y - Y_0) + a_{33}(Z - Z_0) \right\}$$
$$y = f * \left\{ a_{21}(X - X_0) + a_{22}(Y - Y_0) + a_{23}(Z - Z_0) \right\} / \left\{ a_{31}(X - X_0) + a_{32}(Y - Y_0) + a_{33}(Z - Z_0) \right\}$$

where $(X - X_0)$, $(Y - Y_0)$ are the rectified photo coordinates at a scale of $f/(Z - Z_0)$ and (x, y) are the photo coordinates (see Fig. 7.1). The matrix A, which makes the photo coordinate axis parallel to the rectified photo coordinates, is given by:

$$A = R_k R_{\phi} R_{\omega} = \begin{pmatrix} a_{11} & a_{12} & a_{13} \\ a_{21} & a_{22} & a_{23} \\ a_{31} & a_{32} & a_{33} \end{pmatrix}$$

Thus,

$$(X - X_o) = (Z - Z_o) * (a_{11}x + a_{21}y + a_{31}f) / (a_{13}x + a_{23}y + a_{33}f)$$
$$(Y - Y_o) = (Z - Z_o) * (a_{11}x + a_{21}y + a_{31}f) / (a_{13}x + a_{23}y + a_{33}f)$$

For example, given (x, y) we can calculate (X - X₀), (Y - Y₀) at Z using X₀, Y₀, Z₀, K, ϕ , ω from airborne GPS.

An aerial photo was scanned and all of its pixels (x, y) were then transformed to pixel $(X - X_0)$, $(Y - Y_0)$. The transformed pixels were then imported to an image analysis software such as ERDAS software to display the rectified photo. ERDAS is an image analysis software which works on Project Vincent. This was tested using campus site photos and found to be satisfactory.

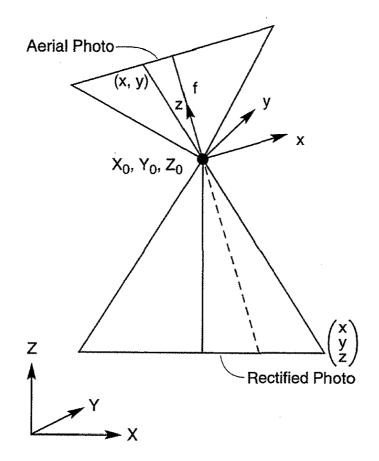


Figure 7.1. Rectification.

7.2. Producing Orthophotos

An orthophoto is a rectified point in which each pixel coordinate $(X - X_0)$, $(Y - Y_0)$ is computed for its own elevation Z_1 using the equation

$$(X - X_o) = (Z_1 - Z_o) * (a_{11}x + a_{21}y + a_{31}f) / (a_{13}x + a_{23}y + a_{33}f)$$
$$(Y - Y_o) = (Z_1 - Z_o) * (a_{11}x + a_{21}y + a_{31}f) / (a_{13}x + a_{23}y + a_{33}f)$$

Thus, the ground elevation for every pixel in the photo must be known. If a contour map exists, then the pixels can be digitized and fed into software such as Arc Info, which will create the Digital Terrain Model for the area. Arc Info will also compute the elevation, Z_1 , of the pixel for given x,y, which can then be used to compute the pixel coordinate of the orthophoto.

Alternatively, if (x_1, y_1) and (x_2, y_2) are the rectified photo coordinates of two adjoining aerial photos at scale $f/(Z - Z_0)$ and $f/(Z - Z_0)$, then both can be brought to the same scale by multiplying the second by a factor of $(Z - Z_0)/(Z - Z_0)$. The elevation z_1 of the pixel is given by:

$$Z_o - Z_1 = B * (x_1 + x_2) / (z_o - z)$$

where

$$x_{2} = x_{2} * (Z_{0} - Z) / (Z_{o} - Z)$$

Z'o is the nodal point coordinate of the second photo (see Fig. 7.2). Thus, if (x_1, y_1) and (x_2, y_2) are known from adjoining photos, then z_1 can be computed. From z_1 the orthophoto coordinate can be computed from the pixel coordinate of each rectified photo. This method was tested using a pair of photos from the campus site and found to be satisfactory.

7.3. Stereo Plotting

In direct stereo plotting after the relative orientation, the following information is required:

- · The base components for scaling
- The ϕ and ω angles for leveling
- The orientation angle, K, and translation (x_0, y_0, z_0) for plotting

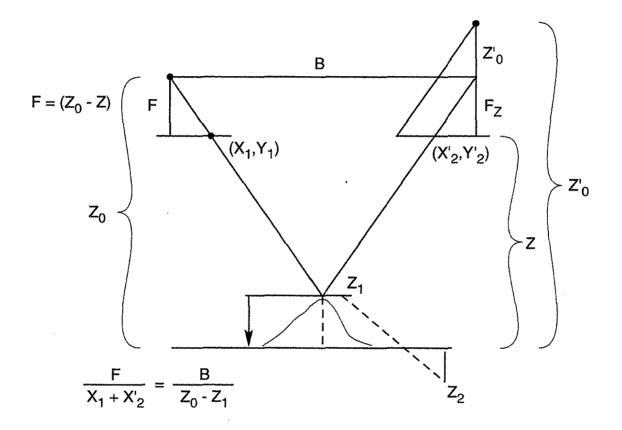


Figure 7.2. Orthophoto production.

.

7.4. Base Components

The base components are computed as follows:

$$Bx = S (X'_0 - X_0)$$
$$By = S (Y'_0 - Y_0)$$
$$Bz = S (z'_0 - z_0)$$

where (x_0, y_0, z_0) and (x'_0, y'_0, z'_0) are obtained from GPS and S is the plotting scale.

7.5. ϕ and ω Angles for Leveling

The leveling angles are obtained from:

 $\omega = \cos K\omega' \pm \sin K \phi' = \omega'$ $\phi = \cos K \phi' \pm \sin K \omega' = \phi'$

where tan $K = (Y_o - Y'_o) / (X_o - X'_o)$ and ω', ϕ' are from GPS.

7.6. Orientation Angle and Translation

The orientation angle is given by:

$$Tan K = \left(Y_o - Y_o\right) / \left(X_o - X_o\right)$$

and the translation is

$$\left(X_o, Y_o\left(Z_o + Z_{om} - Z_m\right)\right)$$

where x_o , y_o , z_o , and x_o , y_o , z_o are from airborne GPS. Z_{om} is the nodal coordinate of the left projector nodal point and Z_m is the model coordinate of a point.

Thus, for small ϕ and ω angles, the nodal coordinates can be transformed to ground coordinates using matrices.

$$\begin{pmatrix} X \\ Y \\ Z \end{pmatrix} = \begin{pmatrix} \cos k & -\sin k & \phi \\ \sin k & \cos & \omega \\ -\phi & \omega & 1 \end{pmatrix} \begin{pmatrix} x_m \\ y_m \\ z_m \end{pmatrix} + \begin{pmatrix} X_o \\ Y_o \\ Z_o \end{pmatrix}$$

Again, this was tested using the campus site photos and the Zeiss Z8 stereo plotter and found to be satisfactory.

When the concept of airborne GPS is accepted and developed fully, further research will be needed for rectification, orthophoto production, and stereo plotting from airborne GPS.

8. CONCLUSIONS AND RECOMMENDATIONS

Airborne GPS is feasible. The coordinates of the camera antenna can be determined with an accuracy better than \pm 10 centimeters if the base reference station is within 10 kilometers of the photographic site, which is acceptable for mapping at all scales.

The PNAV software resolves the integer ambiguity satisfactorily for fast static computation if the rover receiver is within 10 kilometers of the base station.

Camera, wings, and foresight positions are suitable for antenna location; however, the tail is not. The motion of the left and wing antennas are symmetrical and can be used for computing the angle of rotation.

The accuracy of the Z12 GPS receiver is 0.2 millimeters, and the noise due to multipath at the camera, foresight, and wing locations is negligible. The accuracy of the ω obtained from left and right wing antennas at a separation of 10 meters is better than \pm 0.0001 radians, which is acceptable for 2-foot contours using 3,000 feet or lower flying heights.

For a block with more than one strip, no ground control is required. The base station has to be within 10 kilometers of the block, and local geoid undulation has to be applied to the elevations.

For a strip, self calibration is required for transferring ω_G to ω_p and is valid for projects within 10 kilometers. If the project is at a distance greater than 10 kilometers, however, the elevation differences between the two control points separated in the direction perpendicular to the line of flight are required.

Further research is required to obtain ω_p from ω_G with an accuracy of ± 0.00002 radians, an accuracy that GPS is capable of providing. Also, further research is required to use airborne GPS data for rectification, orthophoto production, and direct stereo plotting.

89

9. ACKNOWLEDGMENT

The authors wish to thank the Iowa Highway Research Board for supporting this research project. They appreciate the confidence shown by Mr. George Sisson and Mr. Mel Nutt of the Iowa Department of Transportation (DOT). Thanks are due to Mr. Vernon Marks of the Iowa DOT for his assistance in getting this project completed on time. Thanks also are due to members of the Photogrammetric Section of the Iowa DOT, especially Marlee Walton, Alice Walsh, Roland Popelka, Dennis O'Brien, John Rainey, Jeff Danielson, Mel Holmberg, and others for their unselfish support and assistance during this research period.

Our thanks also go to Rick Hoffman and others from Surdex Inc. for obtaining the aerial photographs for the St. Louis and Ames Project. Mr. Bill Curtrell of Ashtech Inc. obtained airborne GPS data for the projects. Mr. Gary Brown of Aerial Services Inc. supplied the information for evaluating the GPS pinpoint navigation. Dr. Jerry Vogel of the Department of Aerospace Engineering and his students assisted with the analyzing of the aircraft wing motions. We thank them all for their support.

Finally, we also wish to thank Dr. Lowell Greimann, Chairman, and the staff of the Department of Civil and Construction Engineering; Dr. Lord and staff of the Engineering Research Institute; and Iowa State University for their assistance in getting this project completed on time.

10. BIBLIOGRAPHY

- 1. ACSM-NGS. Coordinate Transformation Workshop (unpublished xeroxed paper), ACSM, Falls Church, Virginia, 1987.
- 2. Ashjaee, J. New Results on the Accuracy of the C/A Code GPS Receiver: Proceedings of the First International Symposium on Precise Positioning with the Global Positioning System, U.S. Department of Commerce, Rockville, Maryland, Vol. 1, 207–214. 1985.
- 3. Ashtech Inc. *PRISM II Advanced Software Modules*, Sunnyvale, California, September 1994.
- 4. Ashtech Inc. Ashtech XII Receiver Operations Manual, Sunnyvale, California, 1990.
- 5. Ashtech Inc. Ashtech XII Model L Operating and Processing Manual, Sunnyvale, California, 1989.
- 6. Ashtech Inc. Documentation for Ashtech GPPS Software, Sunnyvale, California, 1989.
- Baker, P. J. Global Positioning System (GPS) Policy: Proceedings of the Fourth International Geodetic Symposium on Satellite Positioning, Sponsored by the Defense Mapping Agency and the National Geodetic Survey at Austin, Texas, Volume 1, 51–64, 1986.
- 8. Bitwise Ideas Inc. Geolab Documentation, Ottawa, Ontario, Canada, 1988.
- 9. Bitwise Ideas Inc. The GPS Environment for Geolab Manual, Ottawa, Ontario, Canada, 1987.
- 10. Bodnar, A. N. Jr. User's Guide for the Establishment of Tidal Bench Marks and Leveling Requirements for Tide Stations, National Geodetic Survey Charting and Geodetic Services National Ocean Service, NOAA, Rockville, Maryland, 1975.
- 11. Bomford, G. Geodesy, 4th ed., Claredon Press, Oxford, 736-739, 1980.
- 12. Bouchard, R. H. Optimized Observation Periods Required to Achieve Geodetic Accuracies Using the Global Positioning System, M. S. thesis, Naval Postgraduate School, Monterey, California, 1988.
- 13. Brown, R. G., and Hwang, P. Y. C. GPS Geodesy: A Kalman Filter Solution to the Wavelength Ambiguity Problem, *Proceedings of the 39th Annual Meeting of the Institute of Navigation*, 87–92, June 20–23, 1983.

- Brown, R. G., and Hwang, P. Y. C. A Kalman Filter Approach to GPS Geodesy, In Global Positioning System, Volume II, Institute of Navigation, Washington, D.C., 155– 166, 1984.
- 15. Brown, R. G., and Hwang, P. Y. C. GPS Geodesy: Real-Time Processing Possibilities with Karman Filter Approach, Institute of Navigation, Washington, D.C., January 1987.
- 16. Brown, R. G., Jeyapalan, K., and Rector, Jack. Use of Global Positioning System for Precise Relative Positioning and Land Surveying, Final Report, Iowa High Technology Council, July 1985.
- 17. Counselman, C. C., and Shapiro, I. I. Miniature Interferometric Terminals for Earth Surveying, *Bulletin Geodesic* 53 (2), 1979.
- 18. Davis, R. E., Foote, F. S., Anderson, J. M., and Mikhail, E. M. Surveying: Theory and Practice, 6th ed., McGraw-Hill, New York, 160–168, 1986.
- 19. Defense Mapping Agency, Department of Defense, *World Geodetic System*, DMA Technical Report 8350.2, Washington, D.C., 3-10, 3-11, 1987.
- 20. Defense Mapping Agency, Department of Defense, *Geodesy for the Layman*, DMA Technical Report 80-003, Washington, D.C., 24, 29, 64, 1983.
- 21. Denker, H., and Wenzel, G. Local Geoid Determination and Comparison with GPS Result, Bulletin Geodesique 61 (4), 349-366, 1987.
- 22. Erck, E. S. Orthometric Height Difference Recovery Tests from GPS Observations and Gravimetry, M.S. thesis, Iowa State University, 1989.
- 23. Ewing, C. E., and Mitchell, M. M. Introduction to Geodesy, New York: Elsevier, 1979.
- 24. Fell, P. J. The Use of Standard Values and Refraction Bias Parameters in Orbit Determination, *The Canadian Surveyor* 29 (3), 301-305, 1975.
- 25. Fell, P. J. Geodetic Positioning Using a Global Positioning System of Satellites, Report No. 289, Dept. of Geodetic Science, Ohio State University, 1980.
- 26. Fury, R. J. Prediction of the Deflections of the Vertical by Gravimetric Methods, NOAA Technical Report NOS NGS 28, 1984.
- 27. Fury, R. J. National Geodetic Survey, U.S. Dept. of Commerce, Rockville, Maryland, Personal communication, 1988.
- 28. Gigierano, J. D. Geological Survey Bureau, Iowa City, Iowa, Personal communication, 1988.

- 29. Heiskanan, W. A., and Moritz, H. *Physical Geodesy*, Graaz, Austria: Institute of Physical Geodesy, Technical University, Reprint, 1984.
- 30. Howell, T. F. Surveying and Mapping in Texas A Case Study Using Automation for Transportation Application, *Surveying and Land Information Systems*, 50 (2), 1990.
- 31. Jeyapalan, K. Photogrammetry, In *Encyclopedia of the Earth System Science*, Volume 3, Academic Press, Inc., 1992.
- 32. Jeyapalan, K. Feasibility Study of the Triangulation of Tanzania, California State University, Fresno, 1979.
- Jeyapalan, K., Ma, Wei-ming, and Tucker, Stevens P. Use of GPS for Precise Prediction of Local Geoid Undulation, ASCM-ASPRS Annual Convention, 78–83, Denver, Colorado, 1990.
- 34. Jeyapalan, K., Stein, M. A., Awuch-Baffour, R., Wang, Jijong, Luzen, B., and Wang, Yan. Use of GPS for Photogrammetry, Iowa State University, Ames, 1992.
- 35. Jeyapalan, K., Bandiman, S., Byrne, M. A., Erick, E. S., and Stein, M. A. Maximized Utility of the Global Positioning System, Iowa State University, Ames 1991.
- 36. Jeyapalan, K. Data Snooping Using Observations and Parameters with Constraints, International Archives of Photogrammetry, Vol. XXXV, 1984.
- 37. Jeyapalan, K. Evaluation of a Prototype Global Positioning System (GPS) Satellite Receiver, ACSM-ASPRS Annual Convention, Volume 2, 1986.
- 38. Jeyapalan, K., and Mohamed, M. The Accuracy Obtainable Using Global Positioning Satellite Systems, ACSM-ASP Annual Convention, Washington, D.C., 1984.
- 39. Jeyapalan, K. Calibration of Comparators by the Method of Collocation, Unpublished report, Topograph Division, U.S. Geological Survey, Reston, Virginia, 2–7, 1977.
- 40. Kaula, W. M. The Need for Vertical Control, Surveying and Mapping 47 (1), 57-64, 1987.
- 41. Kearsley, A. H. W. Tests on the Recovery of Precise Geoid Height Differences from Gravimetry, *Journal of Geophysical Research* 93 (B6), 6559–6570, June 1988.
- 42. King, R. W., Masters, E. G., Rizos, C., and Collins, J. Surveying with GPS, School of Surveying. The University of New South Wales, Kensington, Australia, 128, 1985.
- 43. Lapine, L. A. NOAA Tests Kinematic GPS, ACSM Bulletin 12-14, August 1990.
- 44. Ma, Wei-ming. Local Geoid Determination Using the Global Positioning System, M.S. thesis, Naval Postgraduate School, Monterey, California, 1988.

- 45. Mikhail, E. M. Observations and Least Squares, IEP-Dun-Donnelley, New York, 418–426, 1976.
- 46. Milliken, R. J., and Zoller, C. J. Principle of Operation of NAVSTAR and System Characteristics, *Global Positioning System*, Institute of Navigation, Washington, D.C., Volume 1, 3-14, 1980.
- 47. Moritz, H. Geodetic Reference System 1980, Bulletin Geodesique 54 (3), 395-405, 1980.
- 48. NASA. Directory of Station Locations, 4th ed., Goddard Space Flight Center, Greenbelt, Maryland, 1–11, 1978.
- 49. National Geodetic Survey. *Geodetic Glossary*, Rockville, Maryland: U.S. Government Printing Office, 1986.
- 50. Rapp, R. H., and Cruz, J. Y. Spherical Harmonic Expansions of the Earth's Gravitational Potential to Degree 360 Using 30' Mean Anomalies, Rep no. 376. Department of Geodetic Science and Surveying. The Ohio State University, Columbus, December 1986.
- 51. Rapp, R. H. Department of Geodetic Science and Surveying, The Ohio State University, Columbus, Personal communication, 1988.
- 52. Reilly, J. Surveying with GPS (unpublished xeroxed paper), Presented at ASCE/ICEA Surveying Conference, Ames, Iowa, ASCE/ICEA, 1988.
- 53. Remondi, B. W. Global Positioning System Carrier Phase: Description and Use, Bulletin Geodesique 59 (4), 361–377, 1985.
- 54. Remondi, B. W. Using the Global Positioning System (GPS) Phase Observable for Relative Geodesy: Modeling, Processing and Results, Ph.D. dissertation, The University of Texas at Austin, 1984.
- 55. Rockwell International. Instruction Manual Collins Navcore I GPS C/A Receiver, Cedar Rapids, Iowa, August 1986.
- Strange, W. E., Vincent, S. F., Berry, R. H., and Marsh, J. G. Detailed Gravimetric Geoid for the United States, *The Use of Artificial Satellites for Geodesy*, Geophysical monograph 15. Eds. S. W. Henriksen, A. Mancicni, B. H. Chovitz. Washington: American Geophysical Union, 169–176, 1972.
- 57. Tetley, L., and Calcutt, D. *Electronic Aids to Navigation*, London, Edward Arnold Ltd., 225–232, 1986.
- 58. Torge, W. Geodesy, Berlin: DeGruyter, 1980.
- 59. Trimble Navigation. Trimble Model 40000SX GPS Surveyor-Preliminary-Installation and Operation Manual, Sunnyvale, California, 96, 1987.

- 60. Van Dietendonck, A. J., Russell, S. S., Kopitzke, E. R., and Birnbaum, M. The GPS Navigation Message, In *Global Positioning System*, Institute of Navigation, Washington, D.C., 1980.
- 61. Wells, David G. *Guide to GPS Positioning*, Fredericton, New Brunswick, Canada: University of New Brunswick Graphic Services, 1987.

.

APPENDIX. PROCESSING OF GPS DATA FOR 1994 PROJECT

1. INTRODUCTION

The 1994 project includes two flights. The processing procedures for the two parts are the same. This appendix discusses the procedures of downloading and processing of the data with PNAV software from Ashtech on a PC and Xess on Project Vincent. Then the processing results are given.

2. PROCEDURES

2.1. Downloading Data from the GPS Receivers

Six GPS receivers are used in this project. Two of them are at the ground base stations base 1 (airport south) and base 2 (airport north). The other four receivers are placed on the left wing, right wing, center, and head of the airplane, which are called left, right, camera, and fore respectively in the data processing.

Using a program called HOST in PNAV, all of the data files in the six GPS receivers are downloaded into the hard driver on the PC. In the receivers, different data file names have been given according to the different stations in the flights. You just select Yes or No for each station in the program HOST to decide if it is downloaded. Since there are two flights, we made two directories to store the data of one flight in one directory. The data files from the receivers are B, E, and S files. Photo file is from the receiver on camera.

2.2. Data Processing Using PNAV

To get the positions of left, right, camera, and fore, we use PNAV for the data processing. PNAV performs several task. The input files for data processing in PNAV are B-file (Binary Measurement File) and E-file (Ephemeris File). The S-file (Site Data File) is not used in this release of version. The photo file is used in the procedure of "PHOTOGRAMMETRY" to produce the output for the exposure time.

(a) Run PNAV and select submenu "COMPARE NAV SOLUTIONS" in the main menu "POST MISSIONS." Taking one of the two base stations as base station and the other as rover station, we get the location of the rover station, which is used to compare with the given coordinate of the rover station and find out if the given coordinates of the base stations are correct. The rover motion dynamics for this procedure is "Static."

(b) In data processing, we take base 1, base 2, camera, and left as the base station. Since there are many rover stations for one base, that is, a batch job, we select "BATCH PROCESSING" in the main menu of PNAV. The execution mode is "FORWARD AND BACKWARD." The processing mode is "NAVIGATION" and the rover station dynamics is "STATIC." Each time we select base 1, base 2, camera, and left as the base station and the others as rover stations (except the ground base stations base 1 or base 2). Note that the ground base stations base 1 and base 2 can only be used as base station in the data processing. In this step PNAV creates C-file (rover position in WGS-84 Coordinates) and J-file (relative position in either XYZ or ENU Coordinates) as output files.

(c) After running "BATCH PROCESSING" in the main menu of PNAV, select "POST MISSION" in the main menu. Then run "PHOTOGRAMMETRY" which uses the photo data file to create the position of these receivers at the time of exposures.

(d) The final work done by PNAV is to create P files that are the plot files. Select "CREATE PLOT FILE" in the submenu of "POST MISSION" and input the name of the photo data file.

(e) Select "VIEW PLOT FILE" in the main menu to see the processing results.

2.3. Data Processing Using Xess on Project Vincent

In part two we got the position of left, right, camera, and fore for each flight. In this part for the whole flight we calculate the ω , ϕ , and κ of the aircraft according to these coordinates in the J-file given by PNAV. For those of the exposure time, we get the angles of ω , ϕ , and κ both from the ENU coordinates in J-file and the XYZ coordinates, which are converted from the WGS-84 latitude and longitude coordinates in C-file.

Xess is a spreadsheet software similar to Lotus 1-2-3. We use it to complete the computation for those angles.

(a) For each flight, import the J-file of left, right, camera, and fore that are the results based on base 2 into Xess on Project Vincent. The x, y, and z of each station occupy the adjacent three columns. Then calculate the differences of height, distances, and scale factors between those stations. Finally get the angles ω , ϕ , and k.

(b) To obtain the state plane coordinates from C-file, we use a program "UTM&STPL" to convert the latitude and longitude in C-file. Then repeat the procedure in (a).

3. DATA PROCESSING AND RESULTS

3.1. Data and Results from PNAV

The results data from PNAV are the C-file, J-file, and P-file (Plot file). Viewing the P-file by the "VIEW PLOT FILE" in PNAV, we know that the PDOP are less than two and there are five to seven satellites in the data collecting. In the step "COMPARE NAV SOLUTIONS" we find a error of about 18 meters in the northern direction between base 1 and base 2. The coordinate of

base 2 is regarded as correct one. So we take the results that are based on base 2. The recording interval for the 1994 project is 0.5 second. So the volume of the data is very big and it takes a long time to process all the data by PNAV.

3.2. Data and Results from Xess

There are six sets of data and results from Xess: three for flight 1 and the other three for flight 2.

(a) Data and results from Flight 1

Flight 1 is the high flight on the ISU campus. Only four shots are taken in flight 1. One of them is for the test. So there are relatively less data in flight 1.

Table 1-gives the data of flight 1 for the entire data recording time, exposure time, and data of the SPC converted from WGS-84 in C-file for the exposure time, respectively. Table 2 gives the result from these data.

(b) Data and Results from Flight 2

Flight 2 is the low flight on the ISU campus and on Highway 30. A total of 57 shots occur during flight 2. One is taken on the ground. Four are on the campus. Fifty-one shots are on Highway 30.

4. CONCLUSIONS

From these results, we can reach the following conclusions:

(1) The results from PNAV are good. In the exposure time interval, there are seven satellites. The PDOPs are also very small.

(2) The results from Xess shows that for both flights the distances, three angles, and the scale factors are good. The variances of ϕ and ω for both flights are about 2°.

(3) The results from the ENU coordinates in J-file and the SPC converted from C-file coincide very well, meaning that the results from different methods are consistent.

Table A-1. Data of flight 1 for entire time.

· .

٠

				DATA for FI								
TIME		CAMERA			LEFT			RIGHT			FORE	
	xc	Yc	Zc	Xl	¥1	21	Xr	Yr	Zr	XÍ	ΥĽ	21
				-3111.182								
	-3065.145	2907.787		-3065,833						-3064.156		
	-3019.867			~3020.491								
	-2974.615			-2975.204						-2973.626		
				~2929.961						-2928.395		
	-2884.173									-2883.182		
										-2837.988		
148199.50	-2793.787		951.182	-2794.294			-2793.142			-2792.796		
148200.00	-2748.592			-2749.141							2927.844	
	-2658.203			-2703.956 ~2658.695							2930.738	
	~2612.999			~2613.412							2933.621	
	-2567.779										2936.465	951.788
				-2568.156 -2522.925.								
	-2477.304	2941.008	951.001	~2477.704	2050 013	951.314	-2322.044	2930.200	950.802	-2321.330	2941.861	
148203.50	-2432.051			-2432.442							2947.000	
148204.00	-2386.792	2949.407	951,181	-2387.153			-2386.298			-2385.800		
148204.50				-2341.852						-2340.534		
	-2296.258			-2296.556			-2295.821			-2295.261		
	-2250.985	2956.342					-2250.604				2956.394	
148205.00	-2205.704	2958 417		~2205.905			-2205.363			-2204.705	2958.459	950.984
	-2160.407			-2160.599						-2159,409		
				-2115.305						-2114.105		
	~2069.788	2963.605		-2069.991						-2068.791		951.880
	-2024.464			-2024.697			~2024.093			-2023.466	2965.072	
				~1979.397						-1978.134		
				~1934.061			~1933.388			-1932.794		
	~1888.438			1868 687			~1888.053				2968.899	952.635
	-1843.069			~1843.299	2975.649		-1842.702				2970.063	952.730
	-1797.681			-1797.899			-1797.333			-1796.683	2971.191	
	~1752.283			-1752.455						~1751.285		
	-1706.871			-1707,039			-1706.570			-1705.874	2973.331	
	-1651.445	2974.282		-1661.623	2979.915		-1661.134			-1660.447	2974.319	952.441
	~1616.007	2975.237		-1616.199						~1615.009		
	~1570.559			-1570.770						-1569.560		
148213.50				-1525.307			-1524.746				2977.154	951.663
148214.00	-1479.613			-1479.800		950.776	-1479.291			-1478.615		951.344
148750.00		-1331.563	9.093		-1333.287	8.415		~1330.047	8.430		-1332.518	9,160
148750.50		-1337.256	9,123		-1338.990	8.446		-1335.726	8,459		-1338.208	9.183
148751.00		-1342 828	9,149		-1344.579	8.467		-1341.281	8.493		-1343.781	9.208
148751.50		-1348.276	9,174		-1350.048	8.516		-1346.709	8,497		-1349 227	9.233
148752.00		-1353.594	9,196		-1355.386	8.535		-1352.009	8,521		-1354 545	9.257
148752.50		-1358.784	9.227		-1360.629	8.564		-1357.153	8,567		-1359.733	9.292
148753.00		-1363.846	9.256		-1365.780	8.598		-1362.119	8,587		-1364.788	9.315
148753.50		-1368.774	9.272		-1370.835	8.598		-1366.911	8.623		-1369.709	9.328
148754.00		-1373.561	9.304		-1375.801	8.635		~1371.514	8.541	++	-1374.480	9.358
148754.50		-1378.200	9.325		-1380.355	7.667		-1375.974	8,656		-1379.106	9.375
148755.00		-1382.678	9.322		-1385.281	8.669		-1380.336	8.595		-1383.567	9.377
148755.50		-1387.007	9.351		-1389.784	8.694		-1384.489	8,619	H H - 4	-1387.878	9.400
148756.00		-1391.160	9.368		-1394.099	8.694		-1388.472	8.548		-1392.016	9.420
148756.50		-1395.136	9.382		-1398.228	8.705		-1392.292	8,667		-1395.977	9.436
148757.00		-1398.928	9,390		-1402.179	8 731		-1395.918	8,654		~1399.748	9.446
148757.50		-1402.531	9,406		-1405.987	8,754		~1399.317	8,663		-1403.324	9 459
148758.00		-1405.888	9,427		-1409.588	8,778		-1402.421	8.688		~1406.645	9.472
148758.50		-1408.953	9.429		-1412.923	8.768		-1405.210	8,700		-1409.665	9.475
148759.00		-1411.708	9.424		-1415.976	8.794		-1407.658	8,658		-1412.365	9.464
	~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~ ~~			- 51 555				1	4,000			

.

Table A-2. Results of flight 1 for entire time.

			LEAVETS for PLIT							
	Lo L CHEDOA3	CHECKY CHECK)I	1HI XA	IAL XAPPAR	KAPPAC KAPPAL	* *CALELC	SCALEN	SCALETO	SCALE:	1.040
1 Diff I Dist 1 Diff I Diff I Diff Distance Dist	10332 -5.1043134 7.	4912574 1.3737742	2.4135452 7.314	65763 4. 9071502	8.049009 6.160633	0.99969567 6		1.0019925		
-0.475 -0.184 0.641 -0.109 1.4531414 5.4211306 0.99988540 11.1	102516 -4.8196896 8.	0172503 1,5805851	3.6126358 4.990	32215 1.5431735	7.4434353 7.423925	0.99961147 0	. 91 943054	2.0020356	0 99944TIT	148196.00
-0.495 -0.769 0.063 -0.254 5.4523991 5.6215854 0.99834416 11.5	03961 -5.0740337 7.	6566535 1.3990343	3.6179537 4.336	1204 7.0726684	6.9613624 7.247378	0,99947645 C	. 99948266	1.000497		348197.00
-0.441 -0.776 0.068 -0.295 5.6553401 5.6244898 0.99786833 11.3	08384 -4.8790363 7.	1302843 1,5011952			4.5599954 4.741952		2	1	1	3493\$7.50
-0.441 -0.812 0.043 -0.17 5.4548375 5.4351484 0.55705077 11.3 -0.43 -0.514 0.041 -0.404 5.4571316 5.4341341 0.55843455 11.3					6.3334273 6.614137 6.3509003 6.442733		1.0003244	0.99972018	1.9000724	348198.00
-0.43 -0.434 0.041 -0.404 5.4573336 5.4241241 0.59843455 11.2 -0.472 -0.73 0.035 -0.318 5.4555945 5.4241241 0.99633457 11.2		0268277 1,4151777	3.143744 5.147	2134 4.5751724	5.7590441 5.894461	1.00003311 (1.0007579	0.9999474	148199.50
10.418 -0.812 0.042 -0.414 5.4551039 5.4243999 0.99766427 11.3	209044 -4.3327434 1	9,506806 2,1166703	3.5429509 5.141	4924 6.3851049	3.5795265 5.491941	0.5559999006 (0.99979561	1.0000580	148199.50
-0.191 -0.866 0.058 -0.475 5-6553019 5-6266063 0.9976031 11.5	10154 -3.9445121 8.	-85339924 2,4164593	1.31(0301 3.570	28725 4.9929546	3,3295115 6.319799	0,59999076	1.0001411	0.9991,6204	1.0001669	148200.00
-0.401 -0.641 0.054 -0.44 5.4547802 5.4241291 0.59712164 11.4 -0.401 -0.641 0.057 -0.432 5.4572403 5.4173849 0.99407244 11.4					4.1024239 6.362109 5.4042132 5.695934		1.0002915	1.0003045	1.0002295	148200,50
-0.36 -0.867 0.063 -0.516 5.6573603 5.6179849 0.99607344 11.7 -0.361 -0.867 0.063 -0.516 5.6556636 5.6197774 0.99516078 11.7					4.4209293 4.878976			1.0003043	0.999962886	148201.00
-0.157 -0.907 0.04 -0.55 5.6515403 5.6106973 0.99547203 11.5		2344401 3.8139471	3.4556517 3.826	4213 5.1341047	4.1478907 4.503575	1.0003443 (, \$9*325 73 ·	0.99754657	0. 22263376	\$44302.00
-0.369 -0.879 0.062 -0.51 5.6578478 5.6300916 0.99655105 11.3		\$9\$1\$15 3.4041993	3.3092233 3.020	7431 5.2450946	4.20083 4.50643	1 1-0004045 (2.99932633 ·	0.999666001	0.99990569	248302.50
-0,405 -0.845 0,058 -0.476 5.6537289 5.6204208 0.99675073 11.		4254726 3.3404163	3.3358726 4.051	10515 5.4001511	4.4892383 4.754143	0.99971154	0.9797459	0.89485012	0.99942444	248303.00
-0.429 -0.617 0.06 -0.384 5.654845 5.6210549 0.99579416 11.1	07571 -4.0745471 0		1.4747334 3.4		4.4348266 4.662137 4.1634843 4.275371	0.00001141	0 101111	0.9946071	0. #7977344	168307.50
-0.402 +0.836 0.043 +0.436 5.454871 5.4518023 0.9964783 11.3 -0.35 -0.84 0.043 -0.43 5.454044 5.4203521 0.99431423 11.3					3.7514248 4.014455		199736414	1.0001481	0.99962197	149304,00
	07044 -3.7818105 8.	8722341 2.5343832	3.3232010 3.030	3963 4.4595317	3,4953999 3.760373	Q.99986967 C		1.0016161	0. 23241047	141305.00
-0.397 -0.443 0.057 -0.444 5.4550401 5.4204534 1.000976 11.3	06192 -4.0154347 8,				2.9776099 3.176718			1.0031144		148205.50
	105103 -3.6613163 9. 109731 -3.3077195 9.				3.4024446 3.77250			2,0040054		148306.00
	104513 -3.0419958 9.				2,2900463 2.764561				3.6000154	148304.50
	20852 -2,434741 9.				3.4074416 3.771677			1.003007	1.000013	148207.50
	05557 -3.7235045 8.	-8319406 2.1474445	1.4165319 3.363	13409 3.7837402	3.7496316 3.069844	0.39953677 (. ****55231	1.0020919		140200.00
	207078 -4.1482393 8.	.5500472 2.1635506	1.2671677 2.690	5094 4.0996606	3. 1594333 3.417151	0.99927529	1.99974385	1.0023751		148208.50
	105748 ~4.0773373 8. 209332 ~4.1276524 8.				3.1538277 3.443142 3.4104167 3.342438			1.0011224		249309.00
	06488 -4.0933908 8.				2.4044048 3.055747			1.0036399		166210.00
	04592 -4.2504001 \$.				1.5211462 2.494696		1.000068	1.0024074	1.0000136	\$49710.59
	20714 -4.2723503 8.	3818905 3.040393			2,0057238 2.414017			1.0011554		144211.00
	204928 -4.239194 8				3.1320058 2.398906			1.0013915		140711,50
	105401 -4.3327645 8. 105435 -6.6668338 7.				2,1300058 2.301143			1.0023357	1,0000927	148313.00
	109151 -4.9607533 7.				2.5749451 2.44813			1.003274	1.000122	140217.00
-0.491 -0.738 0.058 -0.147 5.6524346 5.6334569 2.0006963 11.3	04773 -4.9015339 7.	413709 1.2424496	3.3327052 3.24	19409 3.5343403	3.5773453 2.86845	0.99762459		2,0028341	2,0000347	148323.50
	109523 -5.1442425 7.				3.3914453 2.403573	0.99997842	0.9994632	3,0025436	1.0001007	148214.00
		4457874 -0.074422574 45981362 -0.066237135						1.0044009	1.0037438	346750.00
		4172552 -0.13249244					1.0131345	1.0036039	1.0012786	148750.50
		4.133333 0.096434334						1.0041394	1.0071113	149751.50
							1.0113496	1.005\$109	1.0028464	148753.00
		. 6664441 -0. 015388405					1.0107795		1.0020742	146753.50
-0.451 -0.462 0.032 -0.011 3.4106597 5.4906061 1.0018949 11.3 -0.474 -0.449 0.054 0.025 5.434177 5.4471249 1.0045546 11.3		-7511923 0.054047993 -5527077 -0.17741299					1.0117903	1.0040416	1.0021994	146753.00
		4.686378-0.030573723					1.0117917	3.0024948	1.093205	144754.00
	45195 -14. 800142 4.	.7514039 -5.071498	3.8546786-61.70	35564 -45.486583	-44.543359 -44.32404	1,014319	1.0115502	1.005311		148754.50
	177415 -4.6617738 7.				-43.424739 -43.73959		1.0032794		0.9973548	148755.00
	175327 -4.6954959 7.				-40.517354 -41.73490			1.0017084		146755.50
	176619 ~6,8698654 7. 176397 ~6,8892997 7.				-54.644204 -59.77556 -56.432358 -57.43365			1.0045391		144754.00
	43544 -4,7200524 7.				-54,741024 -55,90519		1,0007507	1.0042442	0.994574	148757.00
-0.652 -0.743 0.053 -0.091 5.6320725 5.4240429 1.0030962 11.	16543 -6.647783 7.	.5914159 0.46676434	2.0287335-51.35	57949 -54-06767	-52.237354 -52.32423	0.99500331 4		1.0052391	0.99618552	248757.50
	158436 -6.6199716 7				-48.850319 -50.033			1.0074305		349758.00
		4740485 0.34957748						1.0072961		140750.00
-0.43 -0.766 0.04 -0.136 5.6317950 5.6041471 1.0069125 11.1	143391 -0,4336173 7	4560372 0.69916235	2,378674 = 39,31	******	-40.779044 -41.71896	0.97542374	0.3341433	7.0000131	Q. 99537459	148/39.00